

DATE ISSUED: 6/25/21  
BID DATE: 6/29/21  
PROJECT: Hail Damage Repair – Billings Parmly Library  
ADDRESS: 510 North Broadway, Billings, MT

**ADDENDUM #4:**



Bauer Group Architects PLLC  
P O Box 20939  
Billings, Montana 59104

**GENERAL:**

1. This addendum shall be considered a part of the Contract Documents and shall take precedence over previous requirements for the work. Bidders are required to acknowledge receipt of addendum on the proposal form; failure to do so may result in disqualification.
2. Work not specifically changed or modified by this addendum shall be as originally indicated on the drawings and/or specification.

**CLARIFICATION(S):**

1. Attached are submittal documents for the stainless steel scrim panels and the skylight installations for reference use.

END OF ADDENDUM #4

**ATTACHMENTS:**

Submittal – Scrim  
Submittal - Skylight



O<sup>2</sup> ARCHITECTS

208 N. Broadway - Suite 350 Billings, Montana 59101 TEL (406) 259-7123 FAX (406) 256-7123

**TRANSMITTAL LETTER**

dono@o2architects.net

To: **Jackson Contractor Group**  
**Mike Chase**

Date: 3/27/2013

CC: **WB+P**

Project Name: **Parmly Billings Library**

Project Number: **1101**

We transmit:

( x ) Herewith  
( ) In accordance with your request  
( ) Under separate cover via:

For Your

( ) Approval ( ) Information ( X ) Distribution to Others  
( ) Review & Comment ( ) Record ( ) \_\_\_\_\_

The following:

( ) Drawings ( X ) Shop Drawings ( x ) See Below  
( ) Specifications ( X ) Product Literature ( ) Digital Files

Copies Date: Description:

1 1/21/2013 05 7000 Metal SS Scrim **Approved**

Remarks

## Submittal Packages

Summary with Register Items & Stamp

Parmly Billings Library

510 N. Broadway  
Billings, MT 59101

Project # 2012.35

Jackson Contractor Group Inc.

Tel: 406-542-9150 Fax: 406-542-3515

Item No	Register No	Rev	Spec Section	Sub Section	Description	Responsible	Supplier	Rec'd On	Action
<b>0070 - 05 7000 - 0</b> <b>Ornamental Metalwork - Scrim Channels w/ LEED</b>									
1	00073	0	05 70 00	1.03.A	Ornament. Mtlwork - Drawings	Rollfab Metal Products, LLC	Rollfab Metal Products, LLC	1/16/2013	Submitted
2	00074	0	05 70 00	1.03.B	Ornament. Mtlwork - Prod. Data	Rollfab Metal Products, LLC	Rollfab Metal Products, LLC	1/16/2013	Submitted
3	00075	0	05 70 00	1.03.C	Ornament. Mtlwork - Samples	Rollfab Metal Products, LLC	Rollfab Metal Products, LLC	1/16/2013	Submitted

### SUBMITTAL REVIEW

REVIEWED, NO EXCEPTIONS TAKEN  REVISE AND RESUBMIT  
 NOTE COMMENTS  SEE ATTACHED COMMENTS

Corrections or comments made to the shop drawings during this review do not relieve subcontractor/vendor from compliance with requirements of the drawings and specifications. This check is only for review of general conformance with the design concept of the project and general compliance with the information given in the contract documents. The subcontractor/vendor is responsible for confirming and correlating all quantities and dimensions, selecting fabrication processes and techniques of construction, coordinating his work with that of all other trades, and performing his work in a safe and satisfactory manner.

JACKSON CONTRACTOR GROUP, INC.

BY  MIKE CHASE

DATE  1.21.13

A/E - is there a desired pattern for fasteners?

### SHOP DRAWING / SUBMITTAL REVIEW

APPROVED  APPROVED WITH CHANGES NOTED  
 REVISE & RESUBMIT  REJECTED

SUBMITTAL WAS REVIEWED FOR DESIGN CONFORMITY AND GENERAL CONFORMANCE TO CONTRACT DOCUMENTS ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AT JOB SITES FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF THE WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.

willbruderarchitects **kent mcclure 03.26.13**

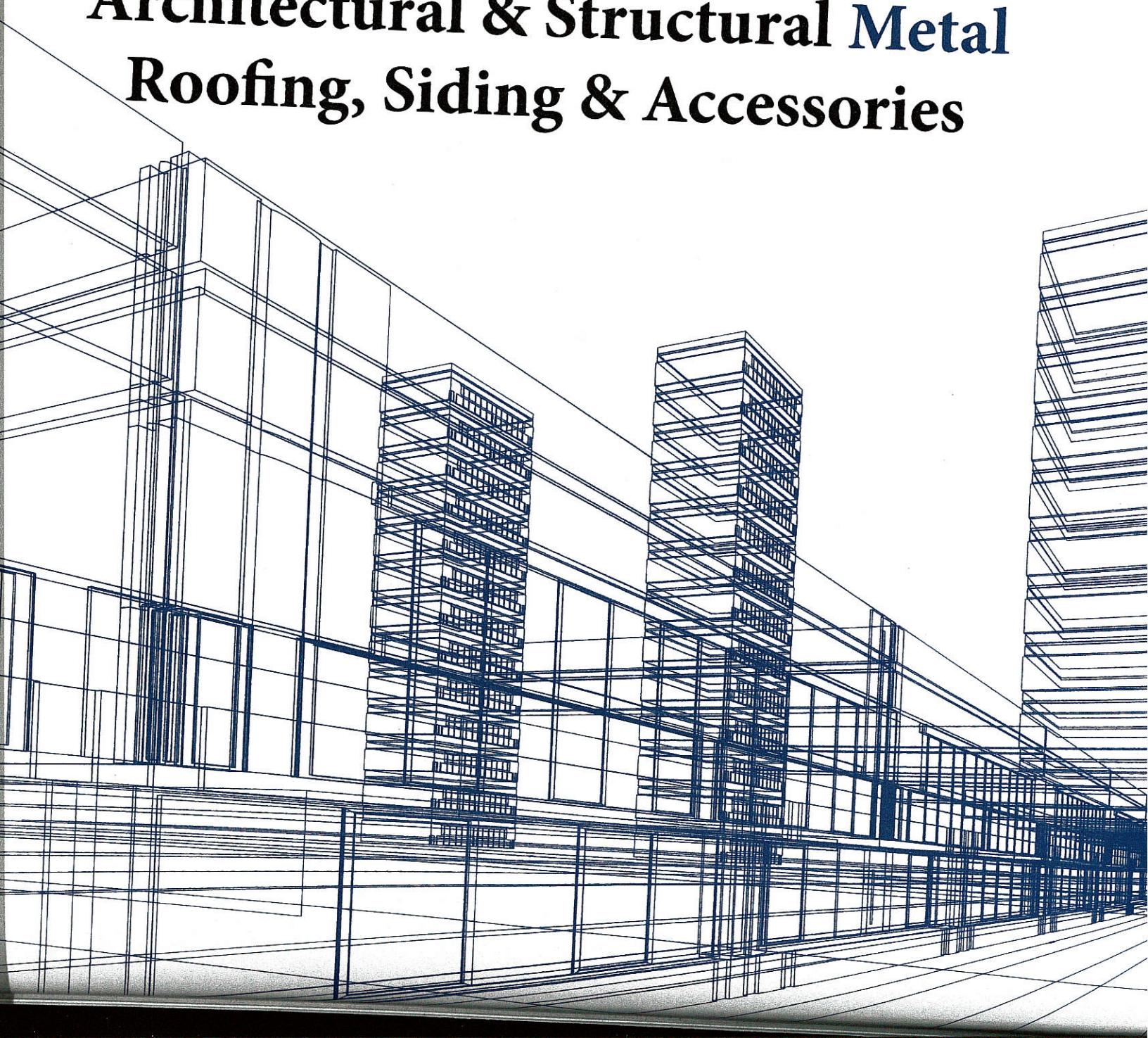
4200 North Central Avenue Phoenix, Arizona 85012 **602.277.5211** f 602.277.5202

# RMP

# ROLLFAB

M E T A L P R O D U C T S

Architectural & Structural Metal  
Roofing, Siding & Accessories



## **MANUFACTURER'S QUALIFICATION STATEMENT**

Rollfab Metal Products was established in 1997 by George Shafer, a veteran manufacturer of prefinished metal roofing and siding in the industry since 1986.

The combined staff of Rollfab has over 50 years of experience in sheet metal fabrication.

Rollfab offers a large selection of architectural metal roofing, metal siding, and custom flashing products for usage on commercial, industrial, and residential projects. These can be provided in various metal types, textures, gauges, finishes, and colors.

Rollfab's 32,000 SF facility located in the heart of Phoenix features "State of the Art" computer controlled manufacturing equipment to achieve the highest quality and consistency for large and small orders. The facility's layout is engineered to minimize the handling of the material and maximize the productive efficiency of the work force. This allows them to produce over a million square feet of product annually.

Rollfab's standing seam roof product offerings are produced on portable roll-forming equipment. This service allows you the design flexibility and construction practicality of having Rollfab "Jobsite Roll-form" panels in excess of shippable lengths. This provides continuous "eave to ridge", or "eave to eave in the case of barrel vault roofs", panel lengths; thus eliminating the need for problematic panel end laps.

Rollfab's technical staff is always ready, available, and willing to render assistance to you in the design, selection, and application of the products you're considering using.

### **Projects of similar size and complexity to Dine College Archival Building:**

*Chandler Performing Arts Center, Chandler, AZ*

*Cocopah Casino in Yuma, AZ*

*Franciscan Winery in Napa Valley, CA*

*Showlow Junior High School, Showlow, AZ*

*U of A Student Housing Center in Tucson*

*Sanctuary on Camelback, Phoenix, AZ*

*Fountain Hills Elementary School, Fountain Hills, AZ*

*The Vale, Tempe, AZ*

*NAU Business Administration Bldg., Flagstaff, AZ*

*Sells Recreation Facility, Sells, AZ*

*Phoenix Fire Station #16, Phoenix, AZ*

*U.S. Bank @ Desert Ridge, Phoenix, AZ*

*Arizona Bank & Trust, Phoenix, AZ*

*Arizona Bank & Trust Chandler, AZ*

*Command and Control Facility Headquarter Fort Bliss, El Paso, TX*

*Apache Junction Magistrate Facility, Apache Junction, AZ*

*Paradise Valley Country Club, Paradise Valley, AZ*

Contact: Steven Tetreault  
Rollfab Metal Building Products  
602-275-1676

Steven Tetreault  
Rollfab Metal Building Products  
602-275-1676

## **CHOOSING AND USING “DESIGNER” METAL ROOFING PANELS**

By Steven Tetreault\*

Picture these scenarios: A commercial client calls to tell you that he wants the look of a copper verdigris roof for a new project... but his budget can't handle the high cost of copper. Another client calls looking for an unusual profile that you can't find in the standard product lines offered by your panel manufacturing sources. Or, a client asks if you can order and install an S-shaped canopy made of curved metal panels.

Would you know how to advise your customers in these situations? The fact is, in recent years, the metal roofing market has grown quite sophisticated, with many more choices available to contractors, architects and building owners. This article will attempt to update you on some of the latest trends in metal roofing, focusing especially on specialty or “designer” panels. The purpose will be to familiarize you with some of the new types of products now available and the processes that make them possible... so that you'll be able to answer the above questions, and more.

In past decades, metal roofing panels were used primarily on metal buildings and rural structures. Though highly functional and low in cost, these panels (mostly corrugated profiles) perpetuated the belief that metal roofing was a low-end product reserved for industrial and agricultural projects.

Today's “designer” panels are a whole different story. Because of their excellent aesthetics, these panels are ideal for commercial and residential applications where corrugated roofing would not have been an option. The result: Your clients can enjoy the high performance and life cycle cost benefits of metal with no compromise in aesthetics.

### **Fabricating Techniques**

Most roofing panels are fabricated using **roll forming** equipment. In this process, sheet metal coil is fed through a machine where it passes under a series of precision rollers that determine the shape of the profile. Every profile requires a different series of rollers; so if a customer wanted to deviate from a standard profile, the cost would usually be prohibitive.

**Press forming** machinery uses top and bottom dies that stamp the sheet metal coil into the desired profile. A system of programmable stops allows the operator to produce the desired angle bend. Using press forming, many different shapes can be made with a single die by simply adjusting the stops. This technique results in greater design flexibility with no need to change tooling.

Advanced **finishing** techniques have also contributed to the growth of designer panels. Today's standard paint coatings offer low maintenance, excellent durability, and a wide palette of colors. In addition, new techniques have made it possible to achieve special design effects. For example, two-toned paint patterns have a "marbled" appearance that lends a richer, more textured look than a standard paint finish for high-end projects. A two-toned pattern is used on the celebrated new Bellagio hotel in Las Vegas, where it simulates the look of copper verdigris using much less costly galvanized steel. The pattern was first created as a painting, which was later replicated with computer imaging techniques and transferred to the machine that would cut the roll. The coil coater then manipulated the pattern on-line until they achieved the desired texture and finish characteristics. Without advanced technologies, printed patterns like this would not be viable.

**Metal curving** techniques have become popular for creating attractively curved roofs, mansards, fascias, canopies and more. Probably the most versatile technique is **crimp-curving**, which was introduced to this country in the mid-1980s. Crimp-curving may be used to shape panels into virtually any radius or angle without marring the panel finish, and it is suited to a wide range of profiles from  $\frac{3}{4}$ " to 4" deep in 18-26 GA metal. Crimp-curving can actually double the load factor of steel panels. Crimp-curving creates lightweight yet rigid components that require minimal structural support. The technique may also be used to produce mitered corners and multiple-radius curves – including the "S-shaped" canopy mentioned at the beginning of this story.

**Stretch-forming** is another curving technique that is used on a more limited basis. The stretch-forming equipment pulls the panel around a custom jig or form and stretches it into shape. It provides an alternative in situations where crimp-curving is not viable – for example, when there is a need to bend very deep-ribbed panels. Unlike crimp-curving, the stretching process will decrease the strength of the metal, so a heavier substrate (18-20 GA) is required.

### **Choosing a Standard Profile**

Due to all these advances, for most projects you need look no further than the standard product offerings of today's metal component manufacturers. Before you order a custom panel, check out the following "standard" specialty metal roofing panels:

- Extra-deep profiles (panels with a depth of more than three inches) may be selected to create interesting architectural relief effects. Deep-ribbed panels are particularly suited to large-scale industrial or commercial jobs, where their high strength and long-span capabilities make it possible to reduce structural costs. The deep panel ribs are highly aesthetic and may be varied to create different light and shadow effects.

One project recently used a pan and batten roofing system with panel depth of 11 inches! The result was dramatic, to say the least.

- Exposed fastener panels that simulate the look of more costly concealed fastener systems are also available. Their vertical ribs mimic the look of standing seam or batten seam roofing, while simplifying installation and saving money.
- Special modifications can be made to standard panels with no need for special tooling, using press forming machinery as described above. For example, the manufacturer can vary the width of the reveal and/or the top pan to the specified dimensional requirements, creating shadow lines in almost any location on the panel surface. Profiles may be symmetrical or asymmetrical, and ribs may be fabricated in virtually any angle desired. This high degree of flexibility offers greater freedom and control in new construction projects. It can also allow you to reconstruct difficult-to-match panels for retrofit or renovation jobs.
- Tile facsimile panels are designed to combine the high performance of metal with the popular look of clay tile, at just a fraction of the weight. They can therefore be used for residential and commercial projects where tile appearance is desired but weight is a concern. For best performance, look for a system with long-length panels that install vertically from eave to ridge and are secured with screw fasteners. A system with this design will typically offer the fastest installation and greatest wind resistance. Similar products are also available to simulate the look of shingles or shakes.
- Duotone and other high-tech paint finishes can sometimes be used to mimic the look of more expensive substrates or to give panels a textured appearance, as noted earlier.

### **Going Custom**

After all is said and done, if you still decide to go the custom route, be sure to allow ample lead time for design and fabrication of your special panels. Here are some specific considerations to keep in mind:

- How much material will be needed? Most panels are made from coil stock in modules 36", 42" or 48" wide. Panels should be planned with these modules in mind to minimize costly "drop" or waste (the amount of unused coil that will have to be scrapped). The amount of material required by the panel will also affect cost. A deep profile with heavy ribbing will use more metal per square foot than a shallow, flush-faced panel.
- What substrate will be used? The industry standards – galvanized or zinc aluminum coated steel – are the most economical choices, while special materials such as stainless steel or copper will carry a cost premium.
- How complex is the panel design? This may affect cost in other areas, too. A modified panel produced with an existing die will cost less than one requiring special

tooling. A complex design with multiple ribs and angles may be more time-consuming to fabricate than a simpler profile.

- What type of finish will be best? Review the requirements of the project, including climate and environmental conditions, usage, roof slope, etc. Can a standard color be used? Special colors involve a longer lead time (typically 4-8 weeks) and will usually carry minimum order requirements. Should a protective film be applied to protect panels during installation?
- What other fabrication processes will be required? For example, crimp-curving of panels will carry an additional service charge. However, for many applications – such as self-supporting covers or roof decks – the curved panels can actually save money in labor and materials by reducing the amount of structural support needed and allowing you to use a lighter gauge of metal.
- What about code considerations? If the panel is to be used structurally, it must meet load-bearing requirements. If an UL-90 rated panel is modified in any way, its rating will be invalidated. If panels are to be installed over a wood deck, fire codes may be a concern, so check first. Span requirements are generally not a problem.
- What are the panel manufacturer's capabilities? When sourcing custom panels, inquire about the supplier's expertise in this area. Be sure to ask:
  - What types of materials, paint finishes and colors are offered?
  - What are the limitations on panel depths, widths and lengths?
  - What tooling is currently available?
  - Is engineering support available (including calculations and shop drawings)?
  - For curved panels, what type of curving process is used? Again, is technical support available for these applications?

Specialty or “designer” metal panels are not for every application. But it's a good idea to keep informed of the latest developments and understand how to choose and use these new profiles – whether standard or customized. By doing so, you have an opportunity to expand your product base; make inroads into the profitable high end market, and differentiate yourself from competitors.

# # #

*Steven Tetreault is general sales manager for Rollfab Metal Products, Phoenix, AZ., a manufacturer of metal roofing panels. The author can be reached at (602) 275-1676; email [info@rollfabmetal.com](mailto:info@rollfabmetal.com); website at [www.rollfabmetal.com](http://www.rollfabmetal.com)*



# ROLLFAB PERFORATED HAT CHANNEL "A" CUT LIST

## JOB NAME : PARMLEY BILLINGS LIBRARY

JOB NO.: 2202 DETAILLER: SS

VENDOR: RMP

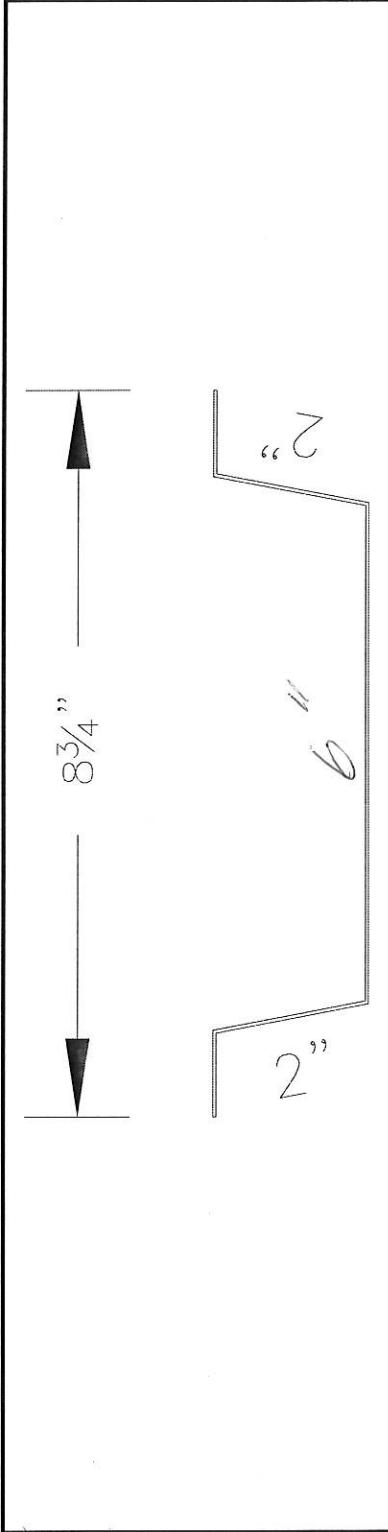
DATE ISSUED: 1/14/13 DATE REQ'D: TBD

MATERIAL: 304-2BSS .127RD x .25STG PERFORATED

MATERIAL DATA	
GAUGE	20

## INPUT PANEL DATA

O.C. SPACING	8.7500
STRETCH OUT	



CUSTOMER: JACKSON CONTRACTING

OWNER: RMP

COLOR: STAINLESS STEEL

QUANTITY	LENGTH FEET	LENGTH INCHES	PROFILE SHAPE	GRID NO.	COMMENTS	LINEAL FEET	NET SQ. FT.	WEIGHT	COMPLETED BY:
<u>324</u>	<u>8 ft</u>	<u>0. in</u>	<u>96.000</u>	<u>A</u>		<u>2592.00</u>	<u>1890.00</u>	<u>0.00</u>	
<u>154</u>	<u>6 ft</u>	<u>0. in</u>	<u>72.000</u>	<u>A</u>		<u>924.00</u>	<u>673.75</u>	<u>0.00</u>	
<u>154</u>	<u>12 ft</u>	<u>0. in</u>	<u>144.000</u>	<u>A</u>		<u>1848.00</u>	<u>1347.50</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>0.313 in</u>	<u>120.313</u>	<u>A</u>		<u>20.05</u>	<u>14.62</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>0.75 in</u>	<u>120.750</u>	<u>A</u>		<u>20.13</u>	<u>14.67</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>2.375 in</u>	<u>122.375</u>	<u>A</u>		<u>20.40</u>	<u>14.87</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>4.875 in</u>	<u>124.875</u>	<u>A</u>		<u>20.81</u>	<u>15.18</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>5.375 in</u>	<u>125.375</u>	<u>A</u>		<u>20.90</u>	<u>15.24</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>5.813 in</u>	<u>125.813</u>	<u>A</u>		<u>20.97</u>	<u>15.29</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>7.125 in</u>	<u>127.125</u>	<u>A</u>		<u>21.19</u>	<u>15.45</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>7.563 in</u>	<u>127.563</u>	<u>A</u>		<u>21.26</u>	<u>15.50</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>8.875 in</u>	<u>128.875</u>	<u>A</u>		<u>21.48</u>	<u>15.66</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>10.188 in</u>	<u>130.188</u>	<u>A</u>		<u>21.70</u>	<u>15.82</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>11.813 in</u>	<u>131.813</u>	<u>A</u>		<u>21.97</u>	<u>16.02</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>1.125 in</u>	<u>133.125</u>	<u>A</u>		<u>22.19</u>	<u>16.18</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>1.563 in</u>	<u>133.563</u>	<u>A</u>		<u>22.26</u>	<u>16.23</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>2.875 in</u>	<u>134.875</u>	<u>A</u>		<u>22.48</u>	<u>16.39</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>4.188 in</u>	<u>136.188</u>	<u>A</u>		<u>22.70</u>	<u>16.55</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>5.813 in</u>	<u>137.813</u>	<u>A</u>		<u>22.97</u>	<u>16.75</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>7.438 in</u>	<u>139.438</u>	<u>A</u>		<u>23.24</u>	<u>16.95</u>	<u>0.00</u>	







# ROLLFAB PERFORATED HAT CHANNEL "A" CUT LIST

JOB NAME : PARMLEY BILLINGS LIBRARY

JOB NO.: 2202

DETAILER : SS

DATE ISSUED: 1/14/13

DATE REQ'D: TBD

VENDOR: RMP

MATERIAL: 304-2B/SS .127RD x 25STG PERFORATED

CUSTOMER: JACKSON CONTRACTING

OWNER: RMP

COLOR: STAINLESS STEEL

2	11 ft	3.25 in	135.250	A			22.54	16.44	0.00
2	11 ft	4.5 in	136.500	A			22.75	16.59	0.00
2	11 ft	5. in	137.000	A			22.83	16.65	0.00
2	11 ft	6.313 in	138.313	A			23.05	16.81	0.00
2	11 ft	6.75 in	138.750	A			23.13	16.86	0.00
2	11 ft	8.375 in	140.375	A			23.40	17.06	0.00
2	11 ft	10.875 in	142.875	A			23.81	17.36	0.00
2	11 ft	11.375 in	143.375	A			23.90	17.42	0.00
2	11 ft	11.813 in	143.813	A			23.97	17.48	0.00
2	6 ft	1.063 in	73.063	A			12.18	8.88	0.00
2	6 ft	1.563 in	73.563	A			12.26	8.94	0.00
2	6 ft	2.875 in	74.875	A			12.48	9.10	0.00
2	6 ft	4.125 in	76.125	A			12.69	9.25	0.00
2	6 ft	5.813 in	77.813	A			12.97	9.46	0.00
2	6 ft	7.438 in	79.438	A			13.24	9.65	0.00
2	6 ft	9.375 in	81.375	A			13.56	9.89	0.00
2	6 ft	11.313 in	83.313	A			13.89	10.12	0.00
2	6 ft	11.813 in	83.813	A			13.97	10.19	0.00
2	7 ft	1.063 in	85.063	A			14.18	10.34	0.00
2	7 ft	1.563 in	85.563	A			14.26	10.40	0.00
2	7 ft	2.875 in	86.875	A			14.48	10.56	0.00
2	7 ft	4.125 in	88.125	A			14.69	10.71	0.00
2	7 ft	5.813 in	89.813	A			14.97	10.91	0.00
2	7 ft	7.063 in	91.063	A			15.18	11.07	0.00
2	7 ft	7.563 in	91.563	A			15.26	11.13	0.00
2	7 ft	8.875 in	92.875	A			15.48	11.29	0.00
2	7 ft	10.125 in	94.125	A			15.69	11.44	0.00
2	7 ft	11.813 in	95.813	A			15.97	11.64	0.00
2	8 ft	0.25 in	96.250	A			16.04	11.70	0.00
2	8 ft	1.938 in	97.938	A			16.32	11.90	0.00
2	8 ft	3.188 in	99.188	A			16.53	12.05	0.00
2	8 ft	4.5 in	100.500	A			16.75	12.21	0.00



# ROLLFAB PERFORATED HAT CHANNEL "B" CUT LIST

JOB NAME: PARMLEY BILLINGS LIBRARY

JOB NO.: 2202 DETAILLER: SS VENDOR: RMP

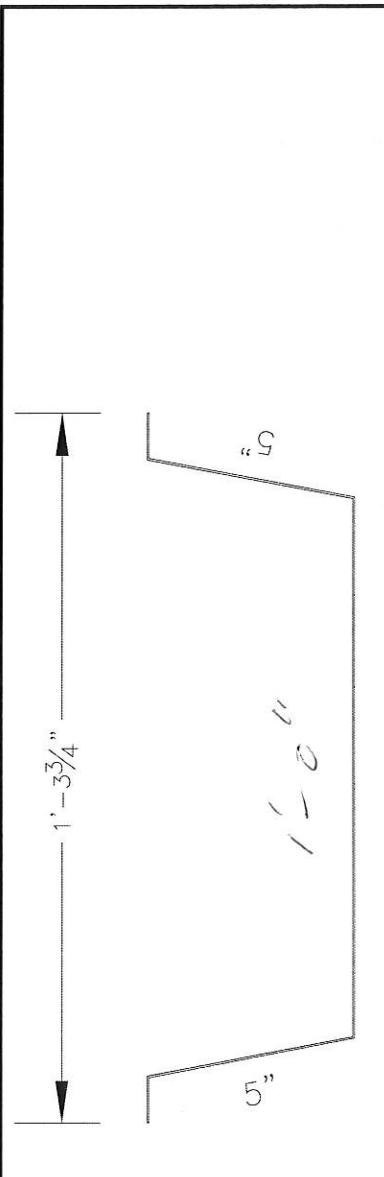
DATE ISSUED: 1/14/13 DATE REQD: TBD MATERIAL: 304-2B/SS .127RD x .25STG PERFORATED

## MATERIAL DATA

GAUGE 20

## INPUT PANEL DATA

O.C. SPACING	<u>15.7500</u>
STRETCH OUT	



CUSTOMER: JACKSON CONTRACTING

OWNER: RMP

COLOR: STAINLESS STEEL

QUANTITY	LENGTH FEET	LENGTH INCHES	PROFILE SHAPE	GRID NO.	COMMENTS	LINEAL FEET	NET SQ. FT.	WEIGHT	COMPLETED BY:
<u>228</u>	<u>8 ft</u>	<u>0. in</u>	<u>96.000</u>	<u>B</u>		<u>1824.00</u>	<u>2394.00</u>	<u>0.00</u>	
<u>112</u>	<u>6 ft</u>	<u>0. in</u>	<u>72.000</u>	<u>B</u>		<u>672.00</u>	<u>882.00</u>	<u>0.00</u>	
<u>104</u>	<u>12 ft</u>	<u>0. in</u>	<u>144.000</u>	<u>B</u>		<u>1248.00</u>	<u>1638.00</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>1.375 in</u>	<u>121.375</u>	<u>B</u>		<u>20.23</u>	<u>26.55</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>3.063 in</u>	<u>123.063</u>	<u>B</u>		<u>20.51</u>	<u>26.92</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>4.188 in</u>	<u>124.188</u>	<u>B</u>		<u>20.70</u>	<u>27.17</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>6.438 in</u>	<u>126.438</u>	<u>B</u>		<u>21.07</u>	<u>27.66</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>8.25 in</u>	<u>128.250</u>	<u>B</u>		<u>21.38</u>	<u>28.05</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>9.5 in</u>	<u>129.500</u>	<u>B</u>		<u>21.58</u>	<u>28.33</u>	<u>0.00</u>	
<u>2</u>	<u>10 ft</u>	<u>11.188 in</u>	<u>131.188</u>	<u>B</u>		<u>21.86</u>	<u>28.70</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>2.25 in</u>	<u>134.250</u>	<u>B</u>		<u>22.38</u>	<u>29.37</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>3.5 in</u>	<u>135.500</u>	<u>B</u>		<u>22.58</u>	<u>29.64</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>5.188 in</u>	<u>137.188</u>	<u>B</u>		<u>22.86</u>	<u>30.01</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>6.438 in</u>	<u>138.438</u>	<u>B</u>		<u>23.07</u>	<u>30.28</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>8.75 in</u>	<u>140.750</u>	<u>B</u>		<u>23.46</u>	<u>30.79</u>	<u>0.00</u>	
<u>2</u>	<u>11 ft</u>	<u>10.375 in</u>	<u>142.375</u>	<u>B</u>		<u>23.73</u>	<u>31.14</u>	<u>0.00</u>	
<u>2</u>	<u>6 ft</u>	<u>1.375 in</u>	<u>73.375</u>	<u>B</u>		<u>12.23</u>	<u>16.05</u>	<u>0.00</u>	
<u>2</u>	<u>6 ft</u>	<u>3.063 in</u>	<u>75.063</u>	<u>B</u>		<u>12.51</u>	<u>16.42</u>	<u>0.00</u>	
<u>2</u>	<u>6 ft</u>	<u>4.188 in</u>	<u>76.188</u>	<u>B</u>		<u>12.70</u>	<u>16.67</u>	<u>0.00</u>	
<u>2</u>	<u>6 ft</u>	<u>6.438 in</u>	<u>78.438</u>	<u>B</u>		<u>13.07</u>	<u>17.16</u>	<u>0.00</u>	

# ROLLFAB PERFORATED HAT CHANNEL "B" CUT LIST

JOB NAME: PARMLEY BILLINGS LIBRARY

JOB NO.: 2202 DETAILER: SS

DATE ISSUED: 1/14/13 DATE REQ'D: TBD

VENDOR: RMP

MATERIAL: 304-2B/SS .127RD x .25STG PERFORATED

CUSTOMER: JACKSON CONTRACTING

OWNER: RMP

COLOR: STAINLESS STEEL

2	6 ft	8.25 in	80.250	B			13.38	17.55	0.00
2	6 ft	9.5 in	81.500	B			13.58	17.83	0.00
2	6 ft	11.188 in	83.188	B			13.86	18.20	0.00
2	7 ft	0.938 in	84.938	B			14.16	18.58	0.00
2	7 ft	2.563 in	86.563	B			14.43	18.94	0.00
2	7 ft	3.875 in	87.875	B			14.65	19.22	0.00
2	7 ft	5.625 in	89.625	B			14.94	19.61	0.00
2	11 ft	0.438 in	132.438	B			22.07	28.97	0.00
2	10 ft	0.438 in	120.438	B			20.07	26.35	0.00
2	10 ft	2.25 in	122.250	B			20.38	26.74	0.00
2	10 ft	3.5 in	123.500	B			20.58	27.02	0.00
2	10 ft	5.188 in	125.188	B			20.86	27.38	0.00
2	10 ft	7.875 in	127.875	B			21.31	27.97	0.00
2	10 ft	9.063 in	129.063	B			21.51	28.23	0.00
2	10 ft	10.688 in	130.688	B			21.78	28.59	0.00
2	11 ft	0.938 in	132.938	B			22.16	29.08	0.00
2	11 ft	2.563 in	134.563	B			22.43	29.44	0.00
2	11 ft	3.875 in	135.875	B			22.65	29.72	0.00
2	11 ft	5.625 in	137.625	B			22.94	30.11	0.00
2	11 ft	6.438 in	138.438	B			23.07	30.28	0.00
2	11 ft	8.25 in	140.250	B			23.38	30.68	0.00
2	11 ft	9.5 in	141.500	B			23.58	30.95	0.00
2	11 ft	11.188 in	143.188	B			23.86	31.32	0.00
2	6 ft	1.375 in	73.375	B			12.23	16.05	0.00
2	6 ft	3.063 in	75.063	B			12.51	16.42	0.00
2	6 ft	4.188 in	76.188	B			12.70	16.67	0.00
2	6 ft	6.438 in	78.438	B			13.07	17.16	0.00
2	6 ft	8.25 in	80.250	B			13.38	17.55	0.00
2	6 ft	9.5 in	81.500	B			13.58	17.83	0.00
2	6 ft	11.188 in	83.188	B			13.86	18.20	0.00
2	7 ft	1.875 in	85.875	B			14.31	18.79	0.00
2	7 ft	3.063 in	87.063	B			14.51	19.04	0.00





**ROLLFAB PERFORATED HAT CHANNEL "B" CUT LIST**

<u>JOB NAME: PARMLEY BILLINGS LIBRARY</u>				<u>CUSTOMER: JACKSON CONTRACTING</u>			
<u>JOB NO.:</u>	<u>2202</u>	<u>DETAILER:</u>	<u>SS</u>	<u>VENDOR:</u>	<u>RMP</u>	<u>OWNER: RMP</u>	
<u>DATE ISSUED:</u>	<u>1/14/13</u>	<u>DATE REQD:</u>	<u>TBD</u>	<u>MATERIAL: 304-2B/SS .127RD x 25STG PERFORATED</u>			
<b>2</b>	<b>8 ft</b>	<b>5.625 in</b>	<b>101.625</b>	<b>B</b>			
						<b>16.94</b>	<b>22.23</b>
							<b>0.00</b>
<b>672</b>						<b>5814.09</b>	<b>7631.00</b>
							<b>0.00</b>

**NOTE: LENGTHS ARE TO BE IN 1/8" MINIMUM INCREMENTS**

# ROLLFAB PERFORATED HAT CHANNEL "C" CUT LIST

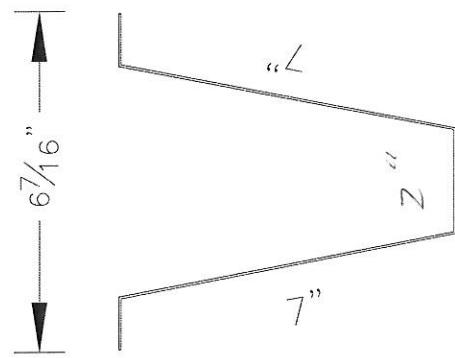
JOB NAME: **PARMLEY BILLINGS LIBRARY**

JOB NO.: **2202** DETAILLER: **SS** VENDOR: **RMP**

DATE ISSUED: **1/14/13** DATE REQD: **TBD** MATERIAL: **304-2B/SS .127RD x .25STG PERFORATED**

MATERIAL DATA	
GAUGE	20

INPUT PANEL DATA	
O.C. SPACING	6.4380
STRETCH OUT	



QUANTITY	LENGTH FEET	LENGTH INCHES	PROFILE SHAPE	GRID NO.	COMMENTS	LINEAL FEET	NET SQ. FT.	WEIGHT	COMPLETED BY:
<b>110</b>	<b>8 ft</b>	<b>0. in</b>	<b>96.000</b>	<b>C</b>		880.00	472.12	0.00	
<b>44</b>	<b>6 ft</b>	<b>0. in</b>	<b>72.000</b>	<b>C</b>		264.00	141.64	0.00	
<b>64</b>	<b>12 ft</b>	<b>0. in</b>	<b>144.000</b>	<b>C</b>		768.00	412.03	0.00	
<b>2</b>	<b>10 ft</b>	<b>2. in</b>	<b>122.000</b>	<b>C</b>		20.33	10.91	0.00	
<b>2</b>	<b>10 ft</b>	<b>3.625 in</b>	<b>123.625</b>	<b>C</b>		20.60	11.05	0.00	
<b>2</b>	<b>10 ft</b>	<b>10.563 in</b>	<b>130.563</b>	<b>C</b>		21.76	11.67	0.00	
<b>2</b>	<b>11 ft</b>	<b>4.563 in</b>	<b>136.563</b>	<b>C</b>		22.76	12.21	0.00	
<b>2</b>	<b>11 ft</b>	<b>7.063 in</b>	<b>139.063</b>	<b>C</b>		23.18	12.43	0.00	
<b>2</b>	<b>11 ft</b>	<b>7.813 in</b>	<b>139.813</b>	<b>C</b>		23.30	12.50	0.00	
<b>2</b>	<b>11 ft</b>	<b>8.188 in</b>	<b>140.188</b>	<b>C</b>		23.36	12.54	0.00	
<b>2</b>	<b>11 ft</b>	<b>9.813 in</b>	<b>141.813</b>	<b>C</b>		23.64	12.68	0.00	
<b>2</b>	<b>11 ft</b>	<b>10.938 in</b>	<b>142.938</b>	<b>C</b>		23.82	12.78	0.00	
<b>2</b>	<b>6 ft</b>	<b>2. in</b>	<b>74.000</b>	<b>C</b>		12.33	6.62	0.00	
<b>2</b>	<b>6 ft</b>	<b>3.625 in</b>	<b>75.625</b>	<b>C</b>		12.60	6.76	0.00	
<b>2</b>	<b>6 ft</b>	<b>10.563 in</b>	<b>82.563</b>	<b>C</b>		13.76	7.38	0.00	
<b>2</b>	<b>7 ft</b>	<b>1.5 in</b>	<b>85.500</b>	<b>C</b>		14.25	7.65	0.00	
<b>2</b>	<b>10 ft</b>	<b>4.563 in</b>	<b>124.563</b>	<b>C</b>		20.76	11.14	0.00	






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- Aggre-gator® 300 Series Stainless Anchors
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- Crete-Flex® SS4 Concrete Anchors
- UltraCon® Concrete Anchors
- Drill Bits (Carbide-Tipped)
- MKT Mechanical Anchors

**For Metal – No Washer:**

- Alumi-Flex™ Stainless Steel Drill Screws
- Bi-Flex™ 300 Series Stainless Drill Screws
- Dril-Flex® Structural Self-Drilling Screws
- Drill Screws, Standard, No Washer
- DrilIt® Wood-to-Metal Reamers
- Fab-Lok® Bolt and Sleeve Fasteners
- Low Profile Screws for Roof Clips
- AllFlex™ Stainless Steel Tapping Screws
- Tap-Flex™ Structural Tapping Screws
- N Rivets – Coming Soon

**For Metal – With Washer:**

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- TapFast® Sealing SDS for Metal-to-Wood
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**Bi-Flex 300 Series SS Fasteners: 12-14 x 2", 5/16" Hex Head, #2 Point, Box of 200**

#12-14 x 2" bi-metal screws with a 300 series stainless steel head and threads fused to hardened steel lead threads and self-drilling point. Stalgard GB coating provides a galvanic barrier. #2 self-drilling point can drill thicknesses up to .140". 5/16" hex drive.

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2128 Springwood • Carrollton, TX 75006 [support@hurripanel.com](mailto:support@hurripanel.com)  
**1.972.417.8882 or toll free 1.877.424.7616**

**Square drive as indicated in spec does not exist**

# RMP

# ROLLFAB

M E T A L   P R O D U C T S

Per Add#4 - Channel to be passivated after  
cutting to prevent rust spotting

## Pro-Finish 500 Color Guide

2529 West Jackson Street

Phoenix, AZ 85009

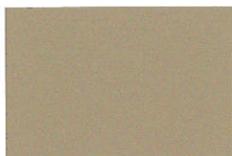
P: (602) 275-1676 F: (602) 275-1739

[www.Rollfabmetal.com](http://www.Rollfabmetal.com)

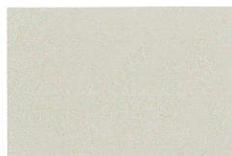
### Pro-Finish 500 Cool Primary Selection<sup>1</sup>



Regal White<sup>4</sup>  
SR: 0.68 • E: 0.86 • SRI: 82



Sierra Tan<sup>4</sup>  
SR: 0.35 • E: 0.86 • SRI: 37



Sandstone<sup>4</sup>  
SR: 0.54 • E: 0.86 • SRI: 63



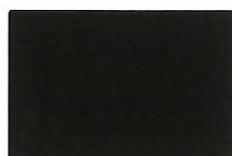
Terra Cotta<sup>4</sup>  
SR: 0.35 • E: 0.87 • SRI: 37



Mansard Brown  
SR: 0.27 • E: 0.86 • SRI: 26



Medium Bronze<sup>4</sup>  
SR: 0.30 • E: 0.87 • SRI: 31



Dark Bronze  
SR: 0.26 • E: 0.84 • SRI: 24



L.G. Colonial Red<sup>4</sup>  
SR: 0.33 • E: 0.85 • SRI: 35



Patina Green<sup>4</sup>  
SR: 0.29 • E: 0.87 • SRI: 29



Evergreen  
SR: 0.27 • E: 0.85 • SRI: 26



Pre Weathered Galvalume<sup>2</sup>  
SR: 0.30 • E: 0.79 • SRI: 27



Copper Metallic<sup>2,4</sup>  
SR: 0.49 • E: 0.85 • SRI: 55



Silver Metallic<sup>2,4</sup>  
SR: 0.60 • E: 0.77 • SRI: 68

### Pro-Finish 500 Cool Accent Selection<sup>1, 3</sup>



Ash Grey<sup>4</sup>  
SR: 0.39 • E: 0.84 • SRI: 41



Slate Grey<sup>4</sup>  
SR: 0.36 • E: 0.84 • SRI: 37



Charcoal Grey  
SR: 0.29 • E: 0.84 • SRI: 28



Hartford Green  
SR: 0.27 • E: 0.85 • SRI: 26



Regal Blue  
SR: 0.26 • E: 0.85 • SRI: 24



Slate Blue  
SR: 0.26 • E: 0.85 • SRI: 24



Brite Red  
SR: 0.42 • E: 0.84 • SRI: 45

Additional Colors: Special Inquiry  
Hemlock Green • Solar White • Stone White  
Regal Red • Champagne Metallic

**Note:**

Longer lead time and added cost may  
apply to accent colors

#### Also Available:

- Zincalume Plus® (acrylic coated Galvalume®)

#### Custom Colors and Small Quantities:

- Available in quantities to a minimum of 1000 lineal feet

#### Oil Canning:

All flat metal surfaces can display waviness or "oil canning" due to standard mill tolerances. Inherent variations in the substrate surface may affect appearances.



#### Notes:

<sup>1</sup> ENERGY STAR qualified products steep slope

<sup>2</sup> Standard mica metallic colors with a modest premium

<sup>3</sup> Additional lead-time and cost may apply

<sup>4</sup> LEED qualified products

*Manufacturers limited paint warranty available at time  
of purchase on Fluorpon® products.*

*Fluorpon® is a registered trademark of the Valspar Corporation*

*Galvalume® is a registered trademark of BIEC International Inc.*

*Zincalume® is a registered trademark of BlueScope Ltd.*

valspar

# RMP

# ROLLFAB

M E T A L   P R O D U C T S

## Pro-Finish 500 Color Guide

2529 West Jackson Street

Phoenix, AZ 85009

P: (602) 275-1676 F: (602) 275-1739

[www.Rollfabmetal.com](http://www.Rollfabmetal.com)

### Performance Specifications

**Specular Gloss at 60° ASTM D 523<sup>2</sup>**

Typical standard gloss: 20-35  
Typical low gloss: 5-15

**Pencil Hardness ASTM D 3363**

HB to 2H

**T-Bend ASTM D 4145<sup>5</sup>**

OT to 3T<sup>3</sup> with no loss of adhesion

**Cross Hatch Adhesion ASTM D 3359**

No loss of adhesion

**Reverse Impact ASTM D 2794<sup>5</sup>**

Galvalume® or HDG: 2x<sup>3</sup> metal thickness inch-pounds  
No loss of adhesion  
Aluminum: 1.5x metal thickness inch-pounds  
No loss of adhesion

**Humidity Resistance**

Galvalume® or HDG: no field blisters  
Aluminum: No field blisters

100% RH 2,000 Hours ASTM D 2247  
100% RH 3,000 Hours ASTM D 2247

**Salt Spray Resistance**

Galvalume® or HDG: Creep from scribe no more than 1/16" (2mm), no field blisters  
Aluminum: Creep from scribe no more than 1/16" (2mm), no field blisters

1,000 Hours ASTM B 117  
3,000 Hours ASTM B 117

**South Florida Exposure**

Film Integrity: 35 years  
Color: No more than 5Δ Hunter units in 30 years  
Chalk: Rating no less than 8 to 30 years

ASTM D 2244  
ASTM D 4214

**Flame Test ASTM E 84**

Class A coating

**Water Immersion 500° hours 100 F ASTM D 870**

No loss of adhesion

**Dew Cycle Weatherometer 1000 hours ASTM D 3361**

Color change: No more than 5Δ Hunter units  
Chalk: Rating no less than 8

**Abrasion Resistance ASTM D 968, Method A**

65 ± 10 liters

**Application Method**

Reverse roll coat

**Substrate<sup>1</sup>**

Aluminum, HDG or Galvalume®

**Viscosity ASTM D 4212 (No. 4 Zahn cup)**

20 to 35 seconds

**VOC (Theoretical) ASTM D 3960**

4.4 to 4.8 pounds per gallon<sup>4</sup>

**Clean-Up Solvent**

Aromatic Ketone blend

**Peak Metal Temperature**

460° to 490 °F

**Flash Point ASTM D 3278**

83° F

**Contains Lubricant**

Yes

**MEK Double Rubs ASTM D 5402**

100 Plus

**Color Side of Panel Dry Film Thickness ASTM D 1005**

Primer: 0.20-0.30 mil  
Top coat: 0.70-0.80 mil  
Total system: 0.90-1.1 mil

**Reverse Side of Panel Dry Film Thickness ASTM D 1005**

Primer: 0.20-0.30 mil  
Top coat: 0.30-0.40 mil  
Total System: 0.50-0.70

**Resin System**

Fluoropolymer system comprised of 70% PVDF resins

**Notes:**

<sup>1</sup> All substrates must be properly pretreated.

<sup>2</sup> American Society for Testing and Materials.

<sup>3</sup> Fluropo is not designed to bridge cracks in the substrate.

<sup>4</sup> Varies by color.

<sup>5</sup> Fluropo coatings will generally meet the requirements for most post-painted fabrication processes. However, variations in metal quality, thickness or cleaning/pretreatment applications can lead to diminished flexibility.

**24 Gauge**

**Description:**  
Cool Pro-Finish 500 is a premium Fluropo® fluoropolymer coating system containing 70% PVDF proprietary resins. Cool Pro-Finish 500 is a field-proven, high performance exterior finish. It provides outstanding resistance to ultraviolet rays, exceptional color retention and resistance to chalking and chemical degradation.

## LEED Information

Rollfab Metal Products uses recycled metal in our roofing components, which are themselves recyclable at the end of their life.

Our standard colors qualify for one credit for the steep slope (>2:12) LEED-NC Credit 2.2 Credit 7.2 Heat Island Effect: Roof in the Sustainable Sites category. The criteria for the points are that the roof must cover at least 75% (excluding skylights and equipment) of the roof surface and have a minimum SRI of 29. In addition, our standard color Regal White will satisfy the SRI value for LOW SLOPE

Rollfab Metal Products uses coil manufactured by a basic oxygen furnace with approximately 25% postconsumer recycled content and 6.6% postindustrial content. This can contribute to the material and resources 4.1 LEED credit.

Other credits to consider when using Rollfab Metal products for this project:

- Contributes to MR Credit for 1.1 or 1.2 – Building Reuse
- Contributes to MR Credit 2.1 or 2.2 – Construction Waste management
- Contributes to EA Credit – Optimize Energy Performance

Regards,

Steven P. Tetreault  
General Sales Manager  
Rollfab Metal Products

*Arizona Owned*

*Arizona Made*

*Arizona Jobs*



## 2008 The Inherent Recycled Content of Today's Steel

This paper provides an overview of the methods used to produce steel in North America today, and describes steel's inherent recycled content. Contemporary technologies produce steel in two ways, both of which require old steel to make new.

The basic oxygen furnace (BOF) process uses 25 to 35 percent old steel to make new. It produces products—such as automotive fenders, encasements of refrigerators, and packaging like soup cans, five-gallon pails, and 55-gallon drums—whose major required characteristic is drawability.

The electric arc furnace (EAF) process uses more than 80 percent old steel to make new. It produces products—such as structural beams, steel plates, and reinforcement bars—whose major required characteristic is strength.

Many are surprised to learn that steel is the world's, as well as North America's, most recycled material, and in the United States alone, almost 75 million tons of steel were recycled or exported for recycling in 2008. This is done for economic as well as environmental reasons. It is always cheaper to recycle steel than to mine virgin ore and move it through the process of making new steel. However, it should also be clearly understood that many steel applications are durables, and even though two out of every three pounds of new steel are produced from old steel, the fact that cars, appliances, and bridges last a long time makes it necessary to continue to mine virgin ore to supplement the production of new steel. Economic expansion, domestically and internationally, creates additional demand that cannot be fully met by available scrap supplies.

Unlike other competing industries, recycled content in the steel industry is second nature. The North American steel industry has been recycling steel scrap for over 170 years through the growth of 2,500 scrap processors and some 12,500 auto dismantlers. Many of them have been in the business for more than 100 years. The pre-consumer, post-consumer, and total recycled content of steel products in the United States can be determined for the calendar year 2008 using information from the American Iron and Steel Institute (AISI), the Institute of Scrap Recycling Industries (ISRI), and

the U.S. Geological Survey. Additionally, a study prepared for the AISI by William T. Hogan, S.A., and Frank T. Koelble of Fordham University is used to establish pre- and post-consumer fractions of purchased scrap.

Individual company statistics are not applicable or instructive because of the open loop recycling capability that the steel and iron industries enjoy, with available scrap typically going to the closest melting furnace. This open loop recycling allows, for example, an old car to be melted down to produce a new soup can, and then, as the new soup can is recycled, it is melted down to produce a new car, appliance, or perhaps a structural beam used to repair some portion of the Golden Gate Bridge.

### Basic Oxygen Furnace

The basic oxygen furnace (BOF) facilities consumed a total of 13,867,000 tons of ferrous scrap in the production of 42,206,000 tons of raw steel during 2008. Based on U.S. Geological Survey statistics, 901,000 of these ferrous scrap tons had been generated as unsalable steel product within the confines of these steelmaking sites. In the steel industry, these tons are classified as "home scrap," but are a mix of runaround scrap and pre-consumer scrap. Estimates by the Steel Recycling Institute identify about 80% of this home scrap as pre-consumer scrap, equating to 721,000 tons (901,000 x 80%). Additionally, these operations reported that they consumed 26,000 tons of obsolete scrap (buildings and warehouses dismantled on-site at the mill) during this time frame. This volume is classified as post-consumer scrap.

<TURN OVER>

For more information, please contact  
the Steel Recycling Institute at  
1-412-922-2772, or visit us online  
at [www.recycle-steel.org](http://www.recycle-steel.org).



As a result of the above, based on the total scrap consumed, outside purchases of scrap equate to 12,940,000 tons [13,867,000 - (901,000 + 26,000)]. According to the Fordham University study, the post-consumer fraction of the purchased ferrous scrap would be 83.4 percent, while 16.6 percent of these purchases would be pre-consumer. This equates to 2,148,000 tons of pre-consumer scrap (12,940,000 x 16.6%). This "prompt scrap" is mainly scrap generated by manufacturing processes for products made with steel.

Therefore, the **total recycled content** to produce the 42,206,000 tons of raw steel in the BOF is:

$$13,867,000 / 42,206,000 = 32.9\% \\ (\text{Total Tons Ferrous Scrap} / \text{Total Tons Raw Steel})$$

Also, the **post-consumer recycled content** is:

$$(12,940,000 - 2,148,000) + 26,000 = 10,818,000 \\ \text{and} \\ 10,818,000 / 42,206,000 = 25.6\% \\ (\text{Post-Consumer Scrap} / \text{Total Tons Raw Steel})$$

Finally, the **pre-consumer recycled content** is:

$$(721,000 + 2,148,000) / 42,206,000 = 2,869,000 / 42,206,000 = 6.8\% \\ (\text{Pre-Consumer Scrap} / \text{Total Tons Raw Steel})$$

## Electric Arc Furnace

The electric arc furnace (EAF) facilities consumed a total of 50,563,000 tons of ferrous scrap in the production of 56,369,000 tons of raw steel during 2008. Based on U.S. Geological Survey adjusted statistics, 16,365,000 of these ferrous scrap tons had been generated as unsalable steel product within the confines of these steelmaking sites. Again, in the steel industry, these tons are classified as "home scrap," but are a mix of runaround scrap and pre-consumer scrap. Estimates by the Steel Recycling Institute identify about 80% of this home scrap as pre-consumer scrap, equating to 13,092,000 tons (16,365,000 x 80%). Additionally, these operations reported that they consumed 77,000 tons of obsolete scrap (buildings and warehouses dismantled on-site at the mill) during this time frame. This volume is classified as post-consumer scrap.

As a result, based on the total scrap consumed, outside purchases of scrap equate to 34,121,000 tons [50,563,000 - (16,365,000 + 77,000)]. According to the Fordham University study, the post-consumer fraction of the purchased ferrous scrap would be 83.4 percent, while 16.6 percent of these purchases would be pre-

consumer. This equates to 5,664,000 tons of pre-consumer scrap (34,121,000 x 16.6%). This "prompt scrap" is mainly scrap generated by manufacturing processes for products made with steel.

Therefore, the **total recycled content** to produce the 56,369,000 tons of raw steel in the EAF is:

$$50,563,000 / 56,369,000 = 89.7\% \\ (\text{Total Tons Ferrous Scrap} / \text{Total Tons Raw Steel})$$

Also, the **post-consumer recycled content** is:

$$(34,121,000 - 5,664,000) + 77,000 = 28,534,000 \\ \text{and} \\ 28,534,000 / 56,369,000 = 50.6\% \\ (\text{Post-Consumer Scrap} / \text{Total Tons Raw Steel})$$

Finally, the **pre-consumer recycled content** is:

$$(13,092,000 + 5,664,000) / 56,369,000 = 18,756,000 / 56,369,000 = 33.3\% \\ (\text{Pre-Consumer Scrap} / \text{Total Tons Raw Steel})$$

The above discussion and calculations demonstrate conclusively the inherent recycled content of today's steel in North America. To buy steel is to "Buy Recycled."

Understanding the recycled content of BOF and EAF steels, one should not attempt to select one steel producer over another on the basis of a simplistic comparison of relative scrap usage or recycled content. Rather than providing an enhanced environmental benefit, such a selection could prove more costly in terms of total life cycle assessment energy consumption or other variables. Steel does not rely on "recycled content" purchasing to incorporate or drive scrap use. It already happens because of the economics. Recycled content for steel is a function of the steelmaking process itself.

After its useful product life, regardless of its BOF or EAF origin, steel is recycled back into another steel product. Thus steel with more than 80 percent recycled content cannot be described as environmentally superior to steel with 30 percent recycled content. This is not contradictory because they are both complementary parts of the total interlocking infrastructure of steelmaking, product manufacture, scrap generation and recycling. The recycled content of EAF relies on the embodied energy savings of the steel created in the BOF. Steel is truly the most recycled material.

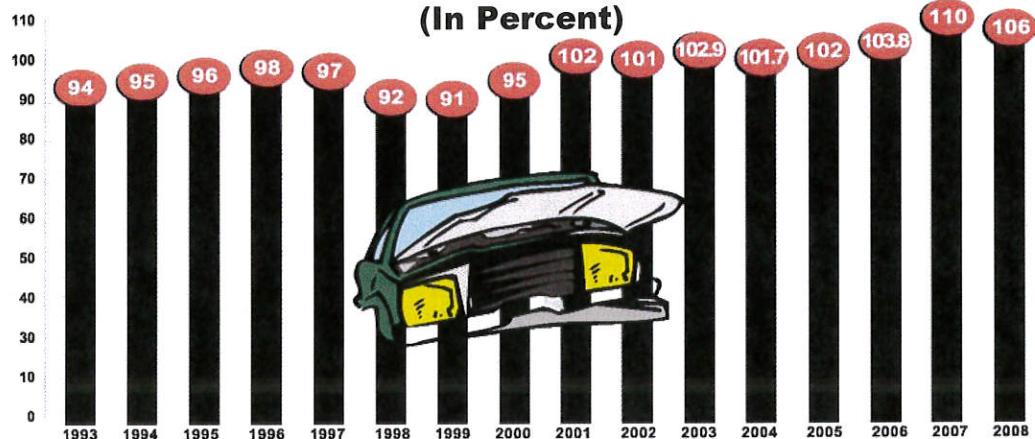


680 ANDERSEN DRIVE • PITTSBURGH, PA 15220-2700 • PHONE: 412.922.2772 • FAX: 412.922.3213 • WEB: WWW.RECYCLE-STEEL.ORG

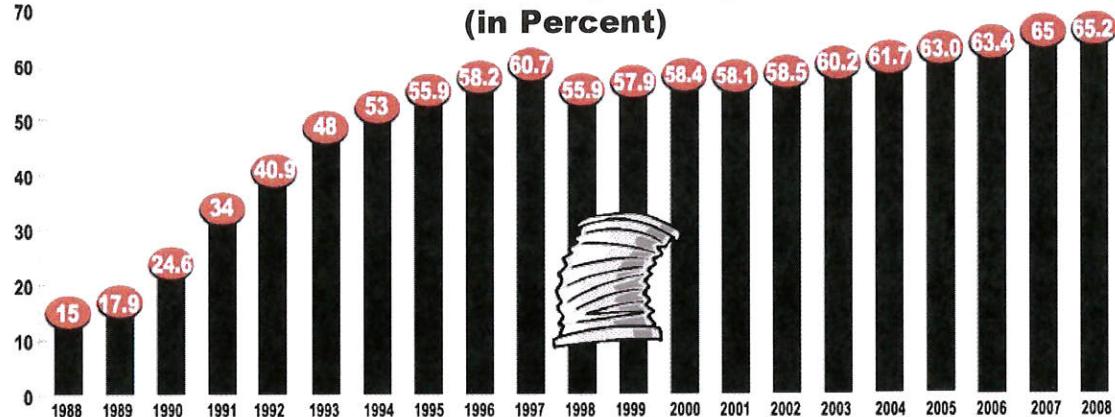
## STEEL RECYCLING RATES AT A GLANCE

### 2008 STEEL RECYCLING RATES

#### Automotive Recycling Rates

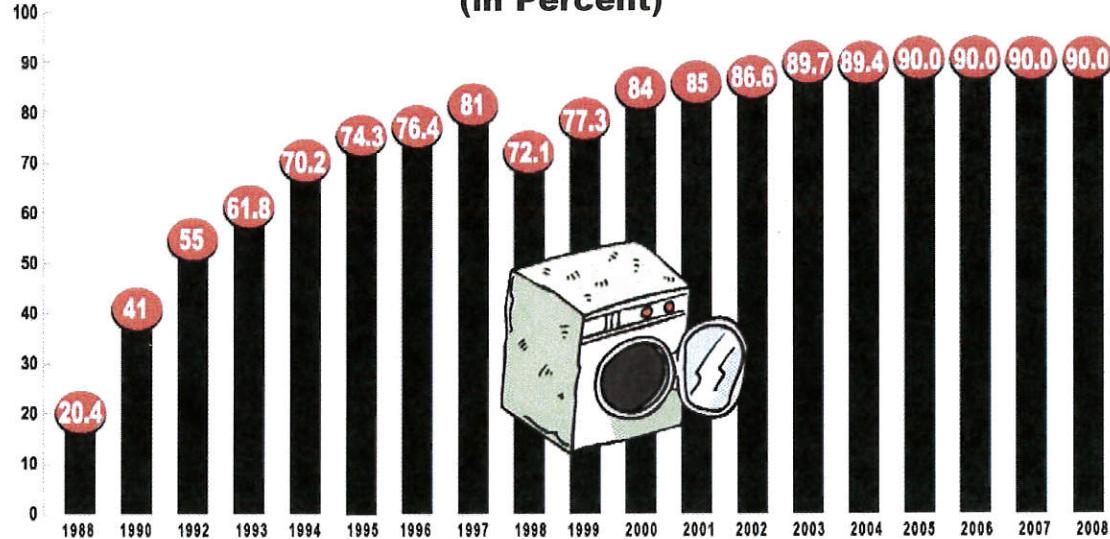


#### Container Recycling Rates



#### Appliance Recycling Rates

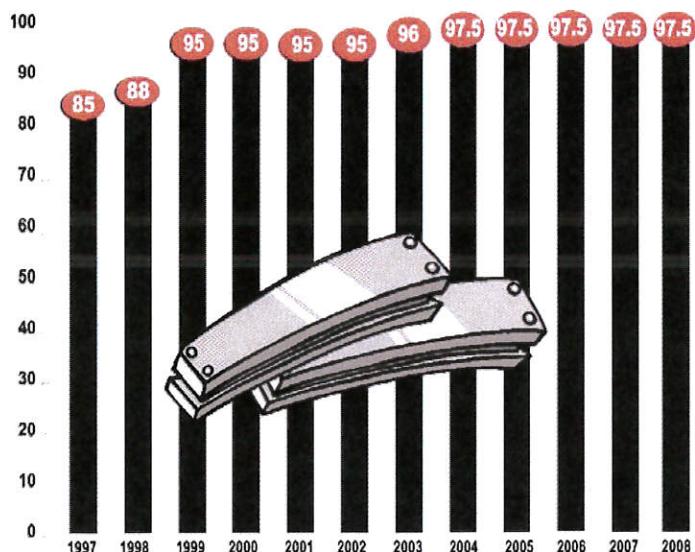
(in Percent)



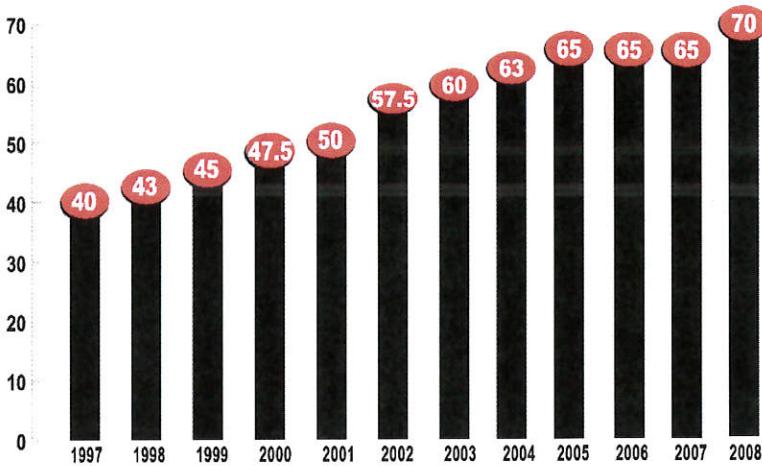
## STEEL RECYCLING RATES AT A GLANCE

## 2008 STEEL RECYCLING RATES

# Construction Structural Recycling Rates (in Percent)

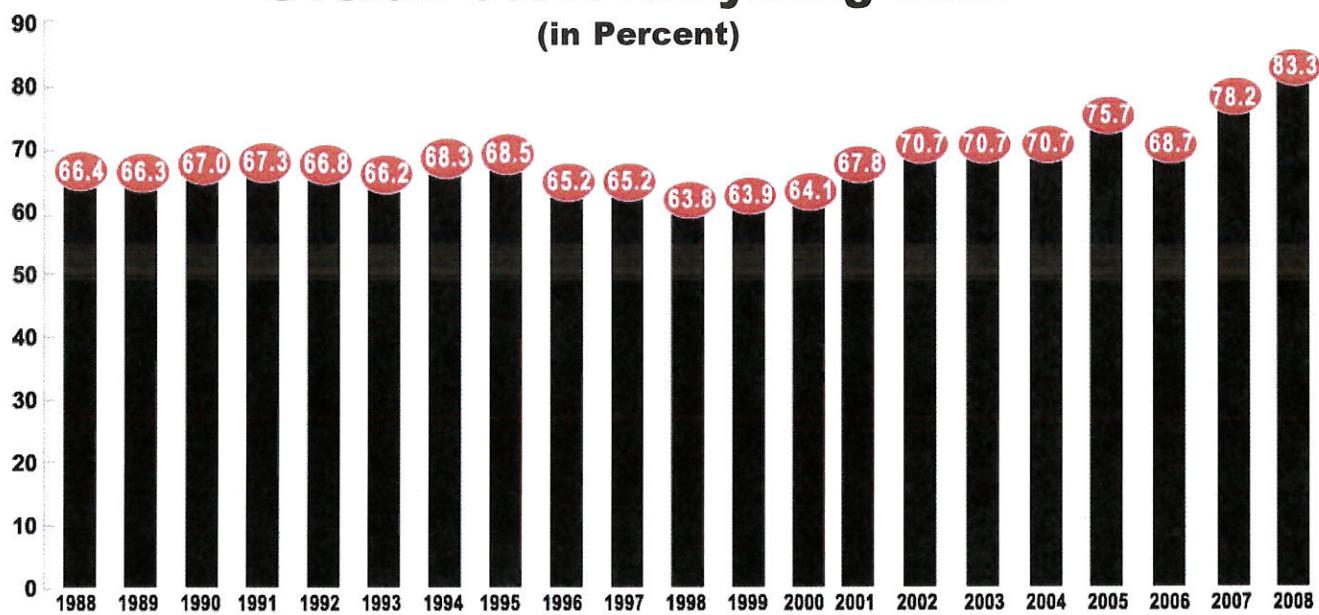


# Construction Reinforcement Recycling Rates (In Percent)



## Overall Steel Recycling Rate

**(in Percent)**





May 2009

RE: Information regarding the "recycled content" of Steelscape's products

Dear valued Steelscape product user:

Steelscape has made a commitment to continually improving its environmental footprint and the sustainability of its products. To that end, we are pleased to present to you some basic information to help inform and educate you on the recycled content of our products.

For background, "recycled content" means the proportion of our products that are generated from post-consumer or pre-consumer material. "Post-consumer" is material generated by households and businesses, such as tin cans and old car bodies. "Pre-consumer" refers to materials recovered from the manufacturing process before it is sold to consumers, such as scrap from the car industry sold back to the steel industry. Reutilization of materials, such as scrap within the steelmaking process is not considered pre-consumer material, and thus not included in calculating recycled content.

In Steelscape's case, because we purchase steel coils for use in our manufacturing process, the recycled content of our steel products is directly related to the recycled content of the steel coils we use. Currently, our main steel supplier uses a basic oxygen furnace (BOF) process with a recycled content of 15-20%. On average, the "pre-consumer" portion accounts for 10-15% and the "post-consumer" portion accounts for 5%.

In accordance with the U.S. Green Building Council LEED recycled content requirements, Steelscape does have the ability to supply a higher recycle content steel upon request. This "high-recycled content" steel contains 50-75% recycled content. On average, the "pre-consumer" portion accounts for 20-35% and the "post-consumer" portion accounts for 30-40%.

These recycled content percentages were calculated using the LEED formula:

Recycled Content = (% post-consumer + ½% pre-consumer)

Steelscape's "high-recycled content" steel is available upon request and must be specified at the time of order entry. Please inquire with your Steelscape Sales Representative.

If you need more information or have further questions please feel free to call me direct.

Very truly yours,  
STEELSCAPE, INC.  
Renee Ramey  
Marketing Manager  
360-673-8236

# Steel Takes LEED® with Recycled Content

## steel beams and columns

## steel studs

## steel roofing

## steel decking

## steel doors

## ductwork

## steel siding

## corrugated steel pipe

## other steel components



Designers and builders have long recognized and lauded steel for its strength, durability, and functionality. Increasingly, however, architects are recognizing steel's important environmental attributes—especially its high recycled content and high reclamation rate.

For many years, there has been a strong economic motive to incorporate recycling into the process for making steel, but today's environmental concerns make recycling even more important. Recycling saves money while conserving energy and resources, as well as reducing solid, liquid, and gaseous wastes. Recycling also helps to spread the energy impact of the original extraction and manufacturing of the material over infinite generations of new steel.

The efficiency with which a material is recycled can be measured by either its *percentage of recycled content* or its *reclamation rate*. Recycled content is a measure of how much recycled material is contained in a finished product. The reclamation rate is a measure of how often a product is actually recycled at the end of its useful life. Steel is an exceptional performer by both measurements. In the construction industry, recent interest in recycling has been driven largely by the U.S. Green Building Council's *Leadership in Energy and Environmental Design* (LEED®) rating system. The LEED rating system only promotes the use of materials with high levels of recycled content. The equally important reclamation rate of the materials is not currently considered.

Scrap consumption in the United States is maximized between the two types of modern steel mills, each of which generates products with varying levels of recycled content. One type of mill produces much of the steel for light flat-rolled steel products with about 30% *recycled content*. The other type of mill makes steel for a wide range of products, including flat-rolled, but is the only method used domestically for the production of structural shapes, which have about 80% *recycled content*. (These processes are covered in detail on the following pages.)

The amount of recycled content in steel products varies over time, both as a function of the cost of steel scrap and its availability. As the world-wide demand for steel increases, the available scrap will be stretched between more and more steel products, meaning that more raw steel will have to enter the production stream to meet the demand. Fortunately, steel is the country's

most widely recycled material, and as more steel is used for construction and other products, more scrap is available for future recycling. At the end of their useful life, about 88% of all steel products and nearly 100% of structural steel beams and plates used in construction are recycled into new products—an amazing reclamation rate!

In addition to recycled content, steel can contribute toward several other LEED credits, either directly or indirectly. Steel is dimensionally stable and, when properly designed, can provide an exceptionally tight building envelope for less air loss and better HVAC performance over time. Steel is made to exact specifications, so on-site waste is minimized. Material from demolition or construction can be easily recycled, with the magnetic properties of steel greatly facilitating its separation from other materials. Thus, in addition to steel's outstanding recycled content and an enviable reclamation rate, steel's other functional properties contribute to the material's solid environmental performance.

As with any building process or material, there are areas for improvement. A great benefit of LEED is that it can help the steel industry recover even more scrap as contractors improve their recycling collection methods at the job site, so less incidental iron and steel scrap escapes to landfills. Similarly, commercial buildings and residential housing can have better disciplined recycling systems for increased recovery.

As steel products reach the end of their useful life, we want to see even more recycled into new steel products for future service to society.

## On-Line Steel Recycling Resources

### [www.recycle-steel.org](http://www.recycle-steel.org)

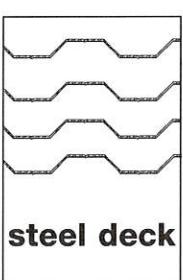
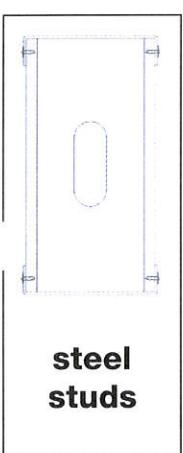
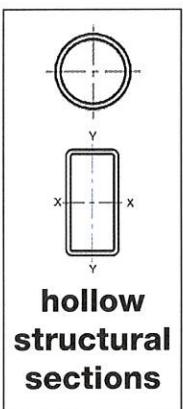
Includes detailed information on recycling rates, recycling databases, and the environmental benefits of steel for homes, buildings, steel roofing, and bridges.

### [www.aisc.org/sustainability](http://www.aisc.org/sustainability)

Includes detailed information on how steel factors into the LEED® rating system, steel mill recycled content documentation, and articles about the use of steel in sustainable projects.

# Modern Steel Production Technologies

## Typical BOF Products



**plate**

**purlins**

Steel is the most recycled material in North America and in the world, and in the United States alone, almost 83 million tons of steel were recycled or exported for recycling in 2007. This is done for economic as well as environmental reasons. It is always cheaper to recycle steel than to mine virgin ore and move it through the process of making new steel. However, it should also be clearly understood that many steel applications are durables, and even though two out of every three pounds of new steel are produced from old steel, the fact that cars, appliances, and bridges last a long time makes it necessary to continue to mine virgin ore to supplement the production of new steel. Economic expansion, domestically and internationally, creates additional demand that cannot be fully met by available scrap supplies.

Unlike other competing industries, recycled content in the steel industry is second nature. The **North American steel industry** has been recycling steel scrap for over 170 years through the growth of 2,500 scrap processors and some 12,500 auto dismantlers. Many of them have been in the business for more than 100 years. The pre-consumer, post-consumer, and total recycled content of steel products in the United States can be determined for the calendar year 2007 using information from the American Iron and Steel Institute (AISI), the Institute of Scrap Recycling Industries (ISRI), and the U.S. Geological Survey. Additionally, a study prepared for the AISI by William T. Hogan, S.A., and Frank T. Koelble of Fordham University is used to establish pre- and post-consumer fractions of purchased scrap.

Individual company statistics are not applicable or instructive because of the open loop recycling capability that the steel and iron industries enjoy, with available scrap typically going to the closest melting furnace. This open loop recycling allows, for example, an old car to be melted down to produce a new soup can, and then, as the new soup can is recycled, it is melted down to produce a new car, appliance, or perhaps a structural beam used to repair some portion of the Golden Gate Bridge.

## Basic Oxygen Furnace

The basic oxygen furnace (BOF) facilities consumed a total of 14,552,500 tons of ferrous scrap in the production of 44,503,000 tons of raw steel

during 2007. Based on U.S. Geological Survey statistics, 950,000 of these ferrous scrap tons had been generated as unsalable steel product within the confines of these steelmaking sites. In the steel industry, these tons are classified as "home scrap," but are a mix of runaround scrap and pre-consumer scrap. Estimates by the Steel Recycling Institute identify about 80% of this home scrap as pre-consumer scrap, equating to 760,000 tons ( $950,000 \times 80\%$ ). Additionally, these operations reported that they consumed 10,000 tons of obsolete scrap (buildings and warehouses dismantled on-site at the mill) during this time-frame. This volume is classified as post-consumer scrap.

As a result of the above, based on the total scrap consumed, outside purchases of scrap equate to 13,592,500 tons [ $14,552,500 - (950,000 + 10,000)$ ]. According to the Fordham University study, the post-consumer fraction of the purchased ferrous scrap would be 83.4%, while 16.6% of these purchases would be pre-consumer. This equates to 2,256,400 tons of pre-consumer scrap ( $13,592,500 \times 16.6\%$ ). This "prompt scrap" is mainly scrap generated by manufacturing processes for products made with steel.

Therefore, the **total recycled content** to produce the 44,503,000 tons of raw steel in the BOF is:

$$14,552,500 / 44,503,000 = 32.7\%$$

(Total Tons Ferrous Scrap / Total Tons Raw Steel)

Also, the **post-consumer recycled content** is:

$$(13,592,500 - 2,256,400) + 10,000 = 11,346,100$$

and

$$11,346,100 / 44,503,000 = 25.5\%$$

(Post-Consumer Scrap / Total Tons Raw Steel)

Finally, the **pre-consumer recycled content** is:

$$(760,000 + 2,256,400) / 44,503,000 =$$

$$3,016,400 / 44,503,000 = 6.8\%$$

(Pre-Consumer Scrap / Total Tons Raw Steel)

## Electric Arc Furnace

The electric arc furnace (EAF) facilities consumed a total of 57,199,300 tons of ferrous scrap in the production of 61,329,700 tons of raw steel during 2007. Based on U.S. Geological Survey adjusted statistics, 15,403,700 of these ferrous scrap tons had been generated as unsalable steel product within the confines of these steelmaking sites. Again, in the steel industry, these tons are classified as "home scrap," but are a mix of run-around scrap and pre-consumer scrap. Estimates by the Steel Recycling Institute identify about 80% of this home scrap as pre-consumer scrap, equating to 12,323,000 tons ( $15,403,700 \times 80\%$ ). Additionally, these operations reported that they consumed 85,000 tons of obsolete scrap (buildings and warehouses dismantled on-site at the mill) during this time frame. This volume is classified as post-consumer scrap.

As a result, based on the total scrap consumed, outside purchases of scrap equate to 41,710,600 tons [ $57,199,300 - (15,403,700 + 85,000)$ ]. According to the Fordham University study, the post-consumer fraction of the purchased ferrous scrap would be 83.4%, while 16.6% of these purchases would be pre-consumer. This equates to 6,924,000 tons of pre-consumer scrap ( $41,710,600 \times 16.6\%$ ). This "prompt scrap" is mainly scrap generated by manufacturing processes for products made with steel.

Therefore, the **total recycled content** to produce the 61,329,700 tons of raw steel in the EAF is:

$$57,199,300 / 61,329,700 = 93.3\%$$

(Total Tons Ferrous Scrap / Total Tons Raw Steel)

Also, the **post-consumer recycled content** is:

$$(41,710,600 - 6,924,000) + 85,000 = 34,871,600$$

and

$$34,871,600 / 61,329,700 = 56.9\%$$

(Post-Consumer Scrap / Total Tons Raw Steel)

Finally, the **pre-consumer recycled content** is:

$$(12,323,000 + 6,924,000) / 61,329,700 =$$

$$19,247,000 / 61,329,700 = 31.4\%$$

(Pre-Consumer Scrap / Total Tons Raw Steel)

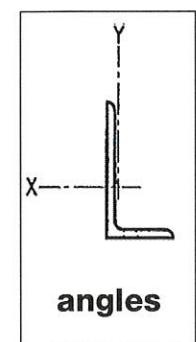
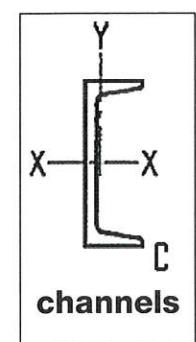
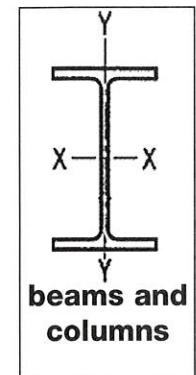
The above discussion and calculations demonstrate conclusively the inherent recycled content of **today's steel in North America**. To buy steel is to "Buy Recycled."

Understanding the recycled content of BOF and EAF steels, one should not attempt to select one steel producer over another on the basis of a simplistic comparison of relative scrap usage or recycled content. Rather than providing an enhanced environmental benefit, such a selection could prove more costly in terms of total life cycle assessment energy consumption or other variables. Steel does not rely on "recycled content" purchasing to incorporate or drive scrap use. It already happens because of the economics. Recycled content for steel is a function of the steelmaking process itself.

After its useful product life, regardless of its BOF or EAF origin, steel is recycled back into another steel product. Thus steel with more than 80% recycled content cannot be described as environmentally superior to steel with 30% recycled content. This is not contradictory because they are both complementary parts of the total interlocking infrastructure of steelmaking, product manufacture, scrap generation and recycling. The recycled content of EAF relies on the embodied energy savings of the steel created in the BOF.

Steel is truly the most recycled material.

## Typical EAF Products



plate

steel deck

piling

## Contact Us

### Steel Recycling Institute

680 Andersen Dr. • Pittsburgh, PA 15220-2700  
412.922.2772 • [sri@recycle-steel.org](mailto:sri@recycle-steel.org)  
[www.recycle-steel.org](http://www.recycle-steel.org)

### American Institute of Steel Construction

One East Wacker Dr., #700 • Chicago, IL 60601  
866.ASK.AISC • [solutions@aisc.org](mailto:solutions@aisc.org)  
[www.aisc.org](http://www.aisc.org)

**To: Architects, Engineers, Designers, and Specifiers**  
**Re: LEED®-NC Version 2.2 and LEED®-NC 2009 Recycled**  
**Content Value of Steel Building Products**

The U.S. Green Building Council Leadership in Energy & Environmental Design (LEED®) Green Building Rating System aims to improve occupant well-being, environmental performance and economic returns of buildings using established and innovative practices, standards, and technologies.



**Steel Recycling Institute**  
680 Andersen Dr.  
Pittsburgh, PA  
15220-2700  
412.922.2772  
sri@recycle-steel.org

**Materials & Resources Credit 4: Recycled Content** intends to increase demand for building products that incorporate recycled content materials, therefore reducing impacts resulting from extraction and processing of new virgin materials. As discussed and demonstrated below, **North American** steel building products contribute positively toward points under Credits 4.1 and 4.2. The following is required by LEED-NC Versions 2.2 and 2009:

**Credit 4.1 (1 point)** "Use materials with recycled content such that the sum of post-consumer recycled content plus one-half of the pre-consumer content constitutes at least 10% (based on cost) of the total value of the materials in the project."

**Credit 4.2 (1 point)** "Use materials with recycled content such that the sum of post-consumer recycled content plus one-half of the pre-consumer content constitutes at least 20% of the total value of the materials in the project."

"The recycled content value of a material assembly shall be determined by weight. The recycled fraction of the assembly is then multiplied by the cost of assembly to determine the recycled content value." Since steel (the material) and steel (the building product) are the same, the value of the steel building product is directly multiplied by steel's recycled content, or:

$$\text{Steel Recycled Content Value} = (\text{Value of Steel Product}) (\text{Post-Consumer \%} + \frac{1}{2} \text{ Pre-Consumer \%})$$

The information contained within this brochure provides post-consumer and pre-consumer recycled content percentages for **North American steel building products**. These percentages and values of steel building products are easily entered into LEED Letter Template spreadsheet for calculation. To illustrate the application of these steel recycled content values to LEED, manual calculations are shown below for typical Basic Oxygen Furnace (BOF) and Electric Arc Furnace (EAF) steel building products with nominal \$10,000 purchases, using 2007 data. Steel building products include steel stud framing, structural steel framing (wide-flange beams, channels, angles, etc.), rebar, roofing, siding, decking, doors and sashes, windows, ductwork, pipe, fixtures, hardware (hinges, handles, braces, screws, nails), culverts, storm drains, and manhole covers.



**American Institute of Steel Construction**  
1 East Wacker Dr.,  
Suite 700  
Chicago, IL  
60601  
866.ASK.AISC  
solutions@aisc.org



**American Iron and Steel Institute**

**American Iron and Steel Institute**  
1140  
Connecticut Ave., Suite 705  
Washington, DC  
20036  
202.452.7100

**BOF Steel Recycled Content Value for Typical Product:**  
**Steel Stud Framing**

Value =  $(\$10,000) (25.5\% + \frac{1}{2} 6.8\%) = (\$10,000) (28.9\%) = \$2,890$   
(Positive net contributor to 10% and 20% goals)

**EAF Steel Recycled Content Value for Typical Product:**  
**Wide-Flange Structural Steel Framing**

Value =  $(\$10,000) (56.9\% + \frac{1}{2} 31.4\%) = (\$10,000) (72.6\%) = \$7,260$   
(Positive net contributor to 10% and 20% goals)



## Submittal Transmittal

Detailed, Grouped by Each Number

<b>Billings Public Library</b> 510 N. Broadway Billings, MT 59101	<b>Project #</b> 2012.35 Tel: 406-542-9150 Fax: 406-542-3515	<b>Jackson Contractor Group Inc.</b>
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**Date:** 1/3/2014 **Reference Number:** 0232

<b>Transmitted To:</b>	Don Olsen O2 Architects 208 N. Broadway Suite 350 Billings, MT 59101 Tel: 406-259-7123 Fax: 406-256-7123	<b>Transmitted By:</b>	Mike Chase Jackson Contractor Group Inc. P.O. Box 967 Missoula, MT 59806 Tel: 406.542.9150 Fax: 406.542.3515
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Qty	Submittal Package No	Description	Due Date	Package Action
1	0054 - 08 6300 - 3	Metal Framed Skylight - ETFE Skylight Package	1/17/2014	Submitted

Transmitted For	Delivered Via	Tracking Number
Approval	e-mail	

Items	Qty	Description	Notes	Item Action
0001	1	Mtl Frame Skylight - Calculations	Calculations only	Submitted
0002	1	Mtl Frame Skylight - Drawings		Submitted
0003	1	Mtl Frame Skylight - Samples	Foil previously approved, mock-up as requested.	Submitted

Cc: Company Name	Contact Name	Copies	Notes
------------------	--------------	--------	-------

Remarks
---------

Final Package for Record

mike chase

1.3.14

Signature

Signed Date

## Submittal Packages

Summary with Register Items & Stamp

**Billings Public Library**  
510 N. Broadway  
Billings, MT 59101

Project # 2012.35

**Jackson Contractor Group Inc.**  
Tel: 406-542-9150 Fax: 406-542-3515

Item No	Register No	Rev	Spec Section	Sub Section	Description	Responsible	Supplier	Rec'd On	Action
<b>0054 - 08 6300 - 3</b> <b>Metal Framed Skylight - ETFE Skylight Package</b>									
0001	00194	3	08 63 00	1.03.A	Mtl Frame Skylight - Calculations	Vectorfoiltec, LLC	Texlon	1/3/2014	Submitted
0002	00195	3	08 63 00	1.03.B	Mtl Frame Skylight - Drawings	Vectorfoiltec, LLC	Texlon	1/3/2014	Submitted
0003	00196	3	08 63 00	1.03.C	Mtl Frame Skylight - Samples	Vectorfoiltec, LLC	Texlon	1/3/2014	Submitted

### SUBMITTAL REVIEW

REVIEWED, NO EXCEPTIONS TAKEN  REVISE AND RESUBMIT  
 NOTE COMMENTS  SEE ATTACHED COMMENTS

Corrections or comments made to the shop drawings during this review do not relieve subcontractor/vendor from compliance with requirements of the drawings and specifications. This check is only for review of general conformance with the design concept of the project and general compliance with the information given in the contract documents. The subcontractor/vendor is responsible for confirming and correlating all quantities and dimensions, selecting fabrication processes and techniques of construction, coordinating his work with that of all other trades, and performing his work in a safe and satisfactory manner.

**JACKSON CONTRACTOR GROUP, INC.**

BY mike chase DATE 1.3.14

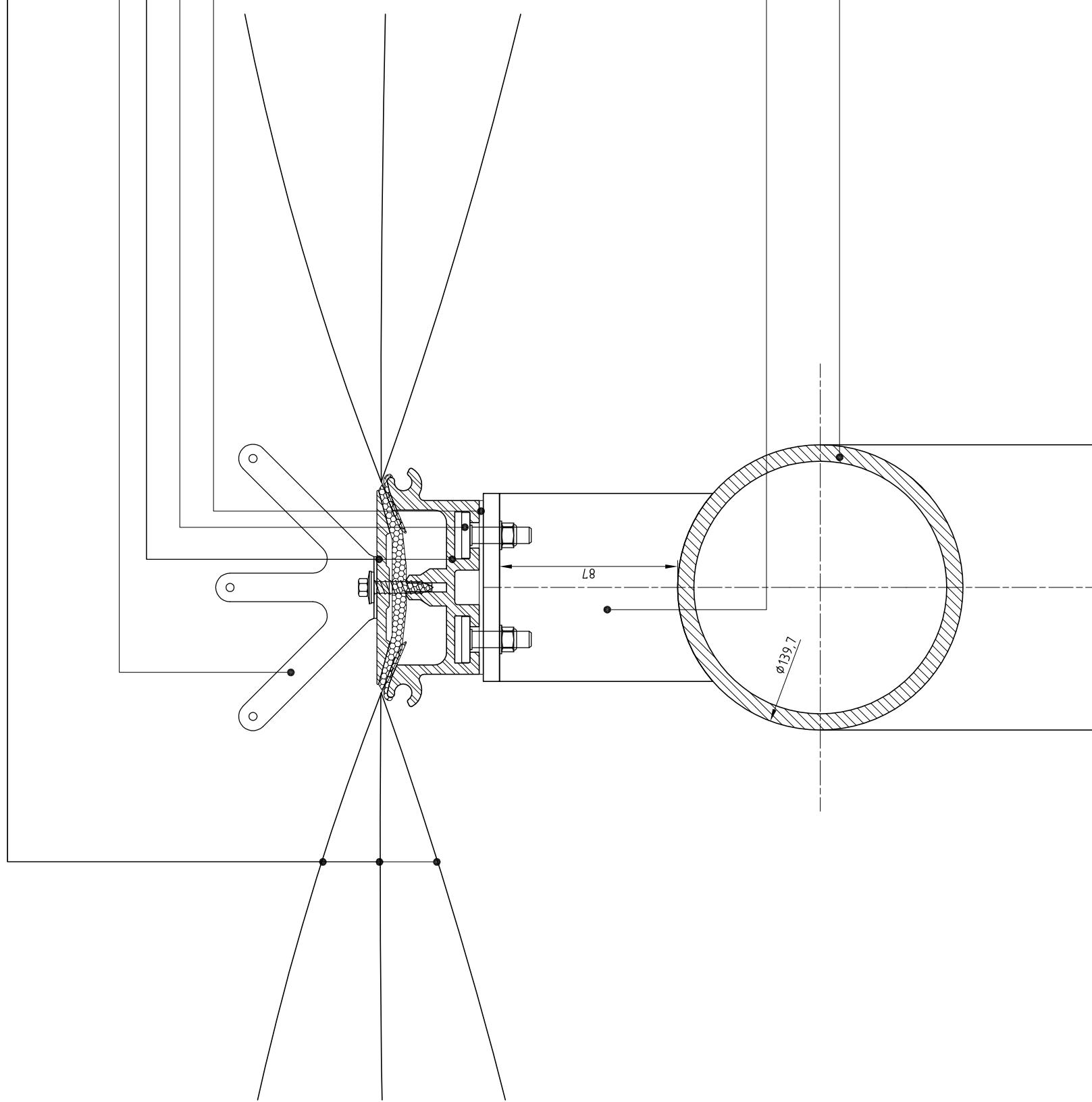
Texlon cushion, 3-layer:  
 top layer 250µm, DM16:50 ink. 31880  
 middle layer 100µm, DR16-4:6:4 ink. 31880  
 bottom layer 200µm, clear  
 (vector foiltec)

bird wire bracket, 3 parged, V2A, e=ca.1500mm (vector foiltec)

Texlon extrusion F16.2, cap + base mill finish (vector foiltec)

hammer head bolt M8x40, V2A (vector foiltec)

EPDM layer, 2mm, black (vector foiltec)



T-upstand, C3 coated, RAL9006, e=ca.1500mm (vector foiltec)

arch beam CHS139,7mm, C3 coated, RAL9006 (vector foiltec)

Project status		Construction	
Scalier Unit	Size	Scalier Unit	Size
Date	Rev	Date	Rev
09.09.2013	C	JE	steel changed according to new roof geometry + structural engineering
26.03.2013	B	JE	steel changed according to structural engineering
22.01.2013	A	LL	amended air supply
Date	Rev	Rev.- Name	Revision
		vector foiltec GmbH	Project status
		Steinacker 3 · D-28717 Bremen	Construction
		Telefon +49-(0)-421-69351-0	
		Telefax +49-(0)-421-69351-19	
		de@vector-foiltec.com	
Drawn by	Date	Checked by	
JE	11/27/2012	XX	US - Billings MT - Parmly Library
Drawn No			
1059	2002		
Rev			
C			

File: G:\PROJECT\US - Billings MT - Parmly Library - 1059\Zeichnungen VF DE\2000\_Details\Details\1059\_2001\_2002\_2003\_RevC\_Detail.dwg  
 Remarks  
 Do not scale from this drawing - All dimensions in millimetres unless otherwise noted. All drawings are copyright and shall not be reproduced without the express consent of Vector Foiltec.

Date: 09.09.2013 14:56:16

Texlon cushion, 3-layer:  
top layer 250µm, DM16:5.0 ink 31880  
middle layer 100µm, DR16-4:6:4 ink 31880  
bottom layer 200µm, clear  
(vector foiltec)

bird wire bracket, 3 ponged, V2A, e=ca.1500mm (vector foiltec)  
Texlon extrusion F16/2, cap + base mill finish (vector foiltec)

hammer head bolt M8x40, V2A (vector foiltec)  
EPDM layer, 2mm, black (vector foiltec)

TPO-foil (vector foiltec)

rigid insulation (vector foiltec)

TPO-coated sheet (vector foiltec)

drain pipe with flexible pipe Ø20mm (vector foiltec)

vapor barrier (vector foiltec)

flashing clip (vector foiltec)

trickle valve VT1, ETFE, Ø25mm (vector foiltec)

T-upstand, C3 coated, RAL9006, every 800-1000mm (vector foiltec)

ring beam CHS139,7mm, C3 coated, RAL9006 (vector foiltec)

arch beam CHS139,7mm, C3 coated, RAL9006 (vector foiltec)

welding seam ring beam on top of column = scope of work (vector foiltec)

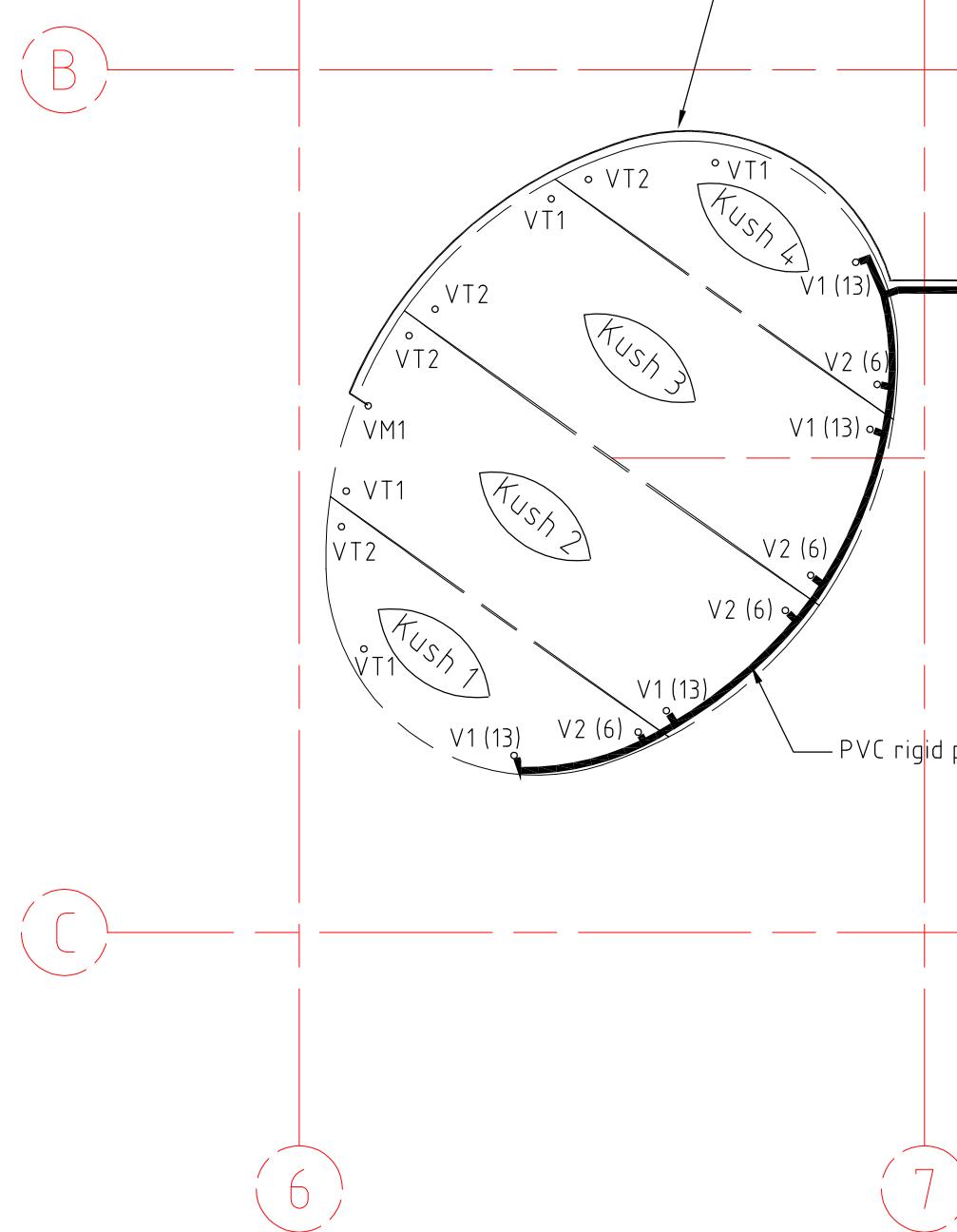
column cut out on site (by others)

09.09.2013 C JE steel changed according to new roof geometry + structural calculation  
26.03.2013 B JE steel changed according to structural calculation  
22.01.2013 A LL amended air supply  
Date Rev. Name Revision

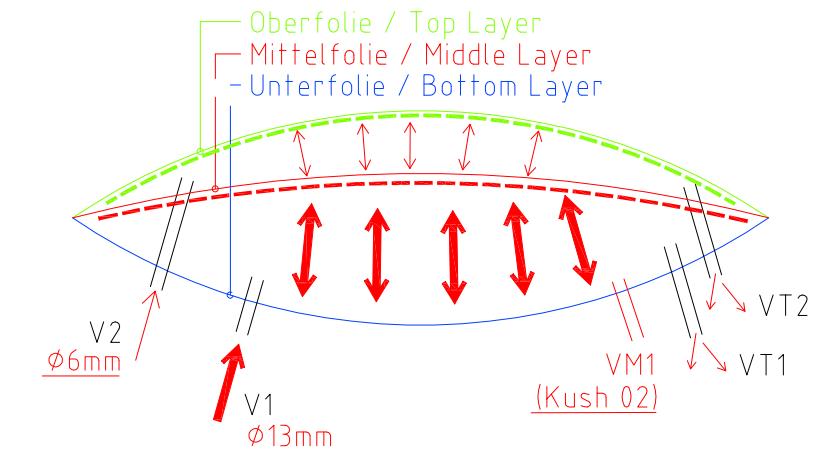
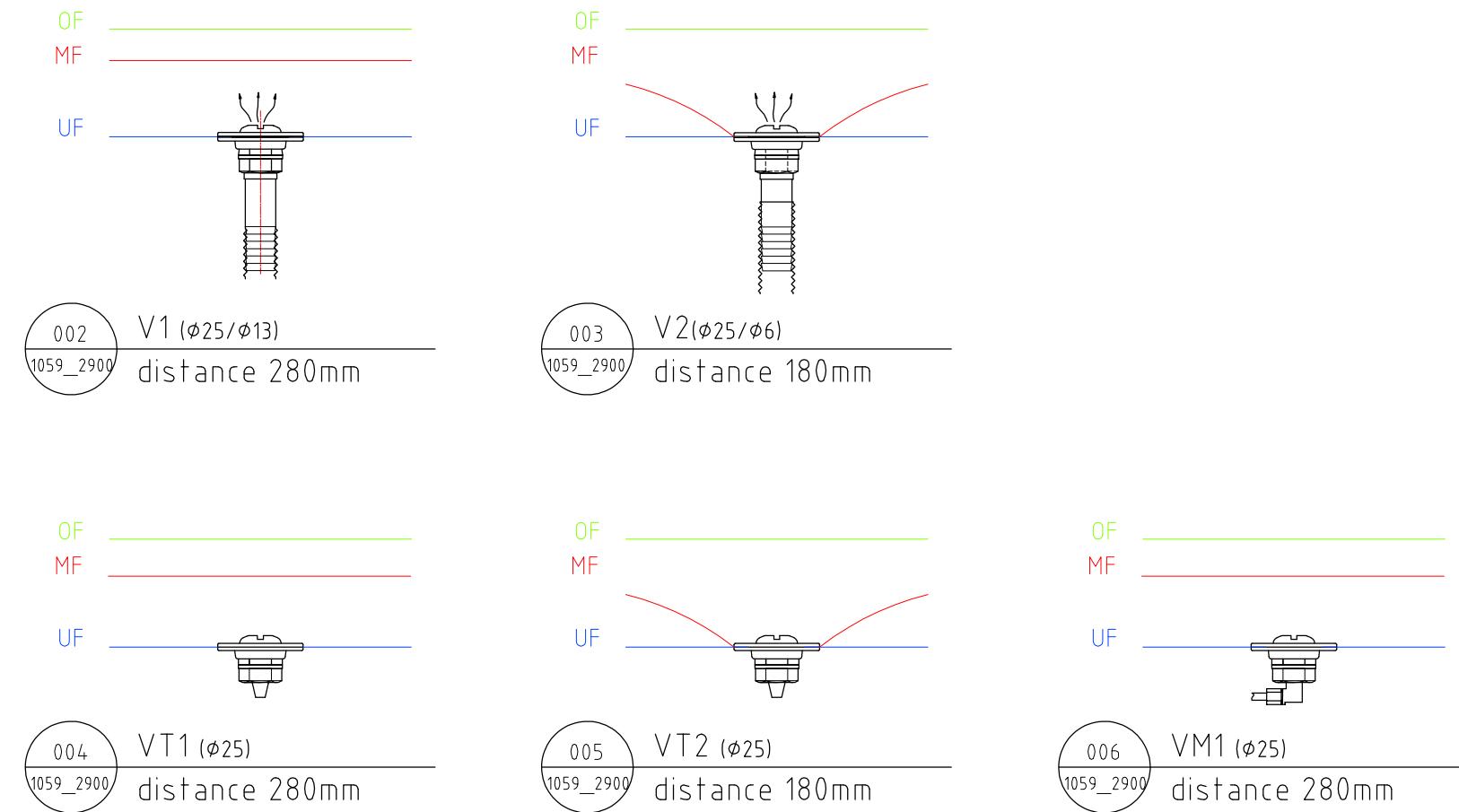
vector foiltec GmbH  
Steinacker 3 · D-28717 Bremen  
Telefon +49-(0)-421-69351-0  
Fax +49-(0)-421-69351-19  
e-mail: de@vector-foiltec.com

Project status  
Construction  
Scale/Unit 1:2,5 mm  
Size A3  
Drawing No 1059\_2003  
Rev C

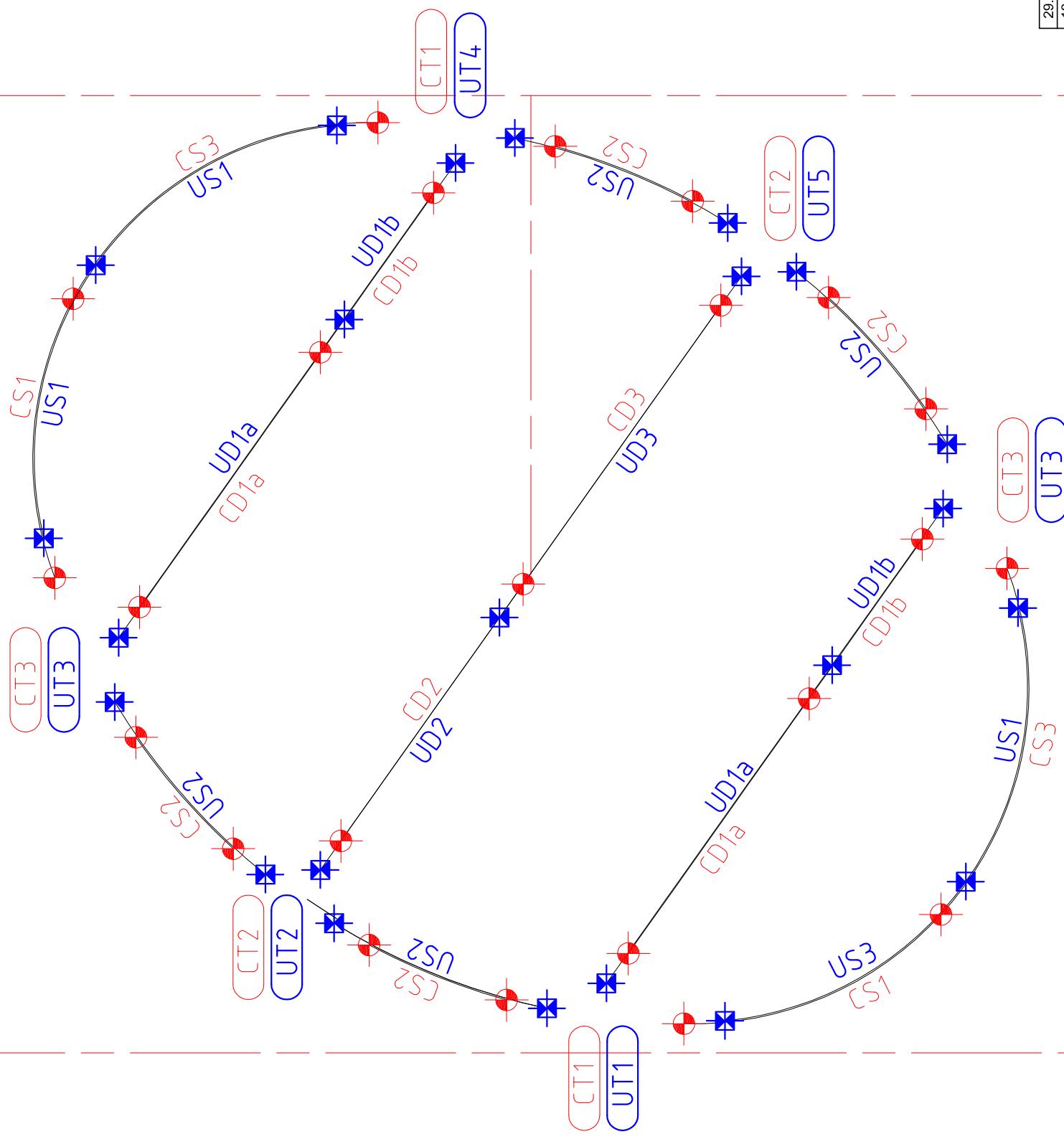
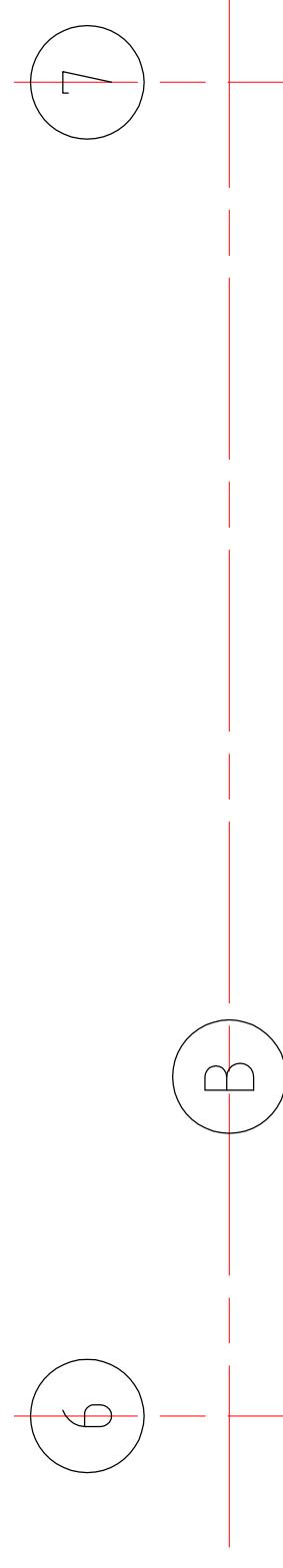
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Date: 09.09.2013 15:12:00  
Remarks  
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001 plan view skylight  
1059\_2900 air supply



08/13/2013	C	JE	general amendments - new roof geometry	Project status
01/23/2013	B	JE	valve position changed	Construction
12/20/2012	A	A. Blohm	amended middle foil	Scale/ Unit
Date	Rev	Rev.- Name	Revision	Size
vector foiltec GmbH				NTS
Steinacker 3 · D-28717 Bremen				A3
Telefon +49-(0)-421-69351-0				
Telefax +49-(0)-421-69351-19				
de@vector-foiltec.com				
Drawn by	Date	Checked by	Project	Drawing No
JE	12/07/2012	XX	US - Billings MT - Parmly Library	Rev
File: G:\PROJEKTE\US - Billings MT - Parmly Library - 1059\Zeichnungen VF DE\2900_Lüfterkiste Inflation unit\1059_2900c_GA air supply.dwg				1059_2900 C
Remarks				Date: 13.08.2013 14:11:52
Do not scale from this drawing - All dimensions in millimetres unless otherwise noted. All drawings are copyright and shall not be reproduced without the express consent of Vector Foiltec.				

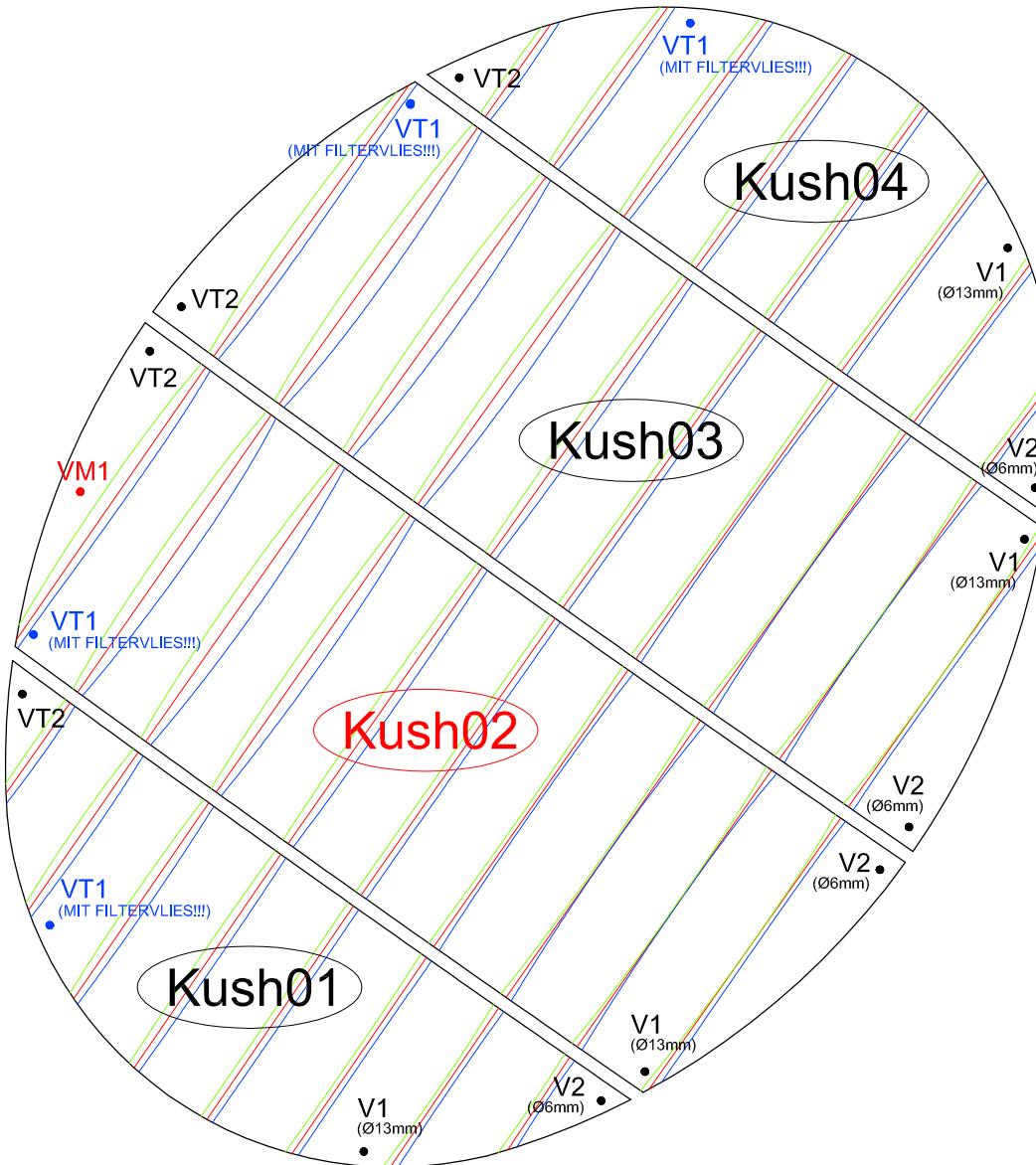


001  
1059\_5000  
General arrangement  
plan view skylight

US - Billings MT - Parmly Library  
XX  
12/11/2012  
JE

File: G:\PROJECT\US - Billings MT - Parmly Library - 1059\Zeichnungen VF DE\5000-6000\_Profile Extrusions\1059\_5000b\_GA\_extrusions\_RevB.dwg  
Remarks  
Do not scale from this drawing - All dimensions in millimetres unless otherwise noted. All drawings are copyright and shall not be reproduced without the express consent of Vector Foiltec.  
Drawing No 1059\_5000  
Rev B  
Date: 29.08.2013 10:07:05

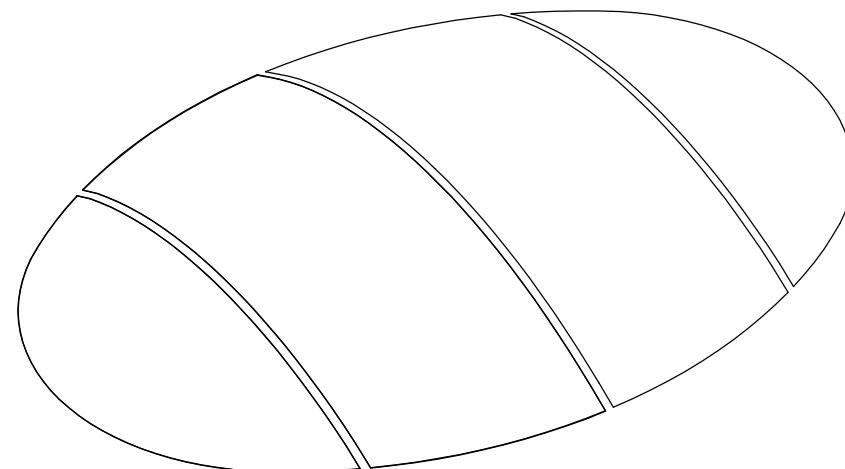
Project status		Construction	
Scale/Unit	NTS	Size	A3
vector foiltec	vector foiltec GmbH	Drawing	
Steinacker 3 · D-28717 Bremen	Steinacker 3 · D-28717 Bremen		
Telefon +49-(0)-421-69351-0	Telefon +49-(0)-421-69351-0		
Telefax +49-(0)-421-69351-19	Telefax +49-(0)-421-69351-19		
de@vector-foiltec.com	de@vector-foiltec.com		
Drawn by	Date	Checked by	Project
JE	12/11/2012		US - Billings MT - Parmly Library
			XX
			1059_5000
			B



Übersicht - Kissen, GA - cushions

001  
1059\_9000

nts



Isometrie, isometric view

002  
1059\_9000

nts

foil make-up:

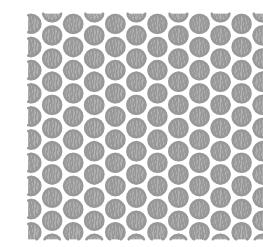
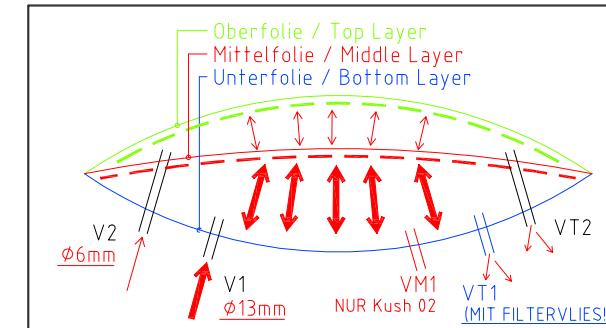
Oberfolie (OF) / top layer (TL):  
Mittelfolie (MF) / middle layer (ML):  
Unterfolie (UF) / bottom layer (BL):

200µm - DM 16:50 ink. 31880  
100µm - DR 16-46:41 ink. 31880  
200µm - clear

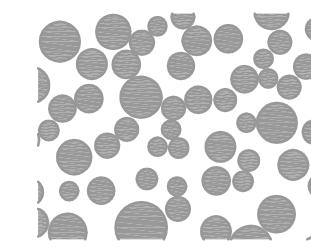
valves:

V1 (Ø25mm, 13mm):  
V2 (Ø25mm, 6mm):  
VT1 (Ø25mm) **MIT FILTERVLIES!!!**  
VT2 (Ø25mm):  
VM1 (Ø25mm) **NUR Kush 02:**

distance from foiledge 280mm  
distance from foiledge 180mm  
distance from foiledge 280mm  
distance from foiledge 180mm  
distance from foiledge 280mm



DM 16:50 ink. 31880



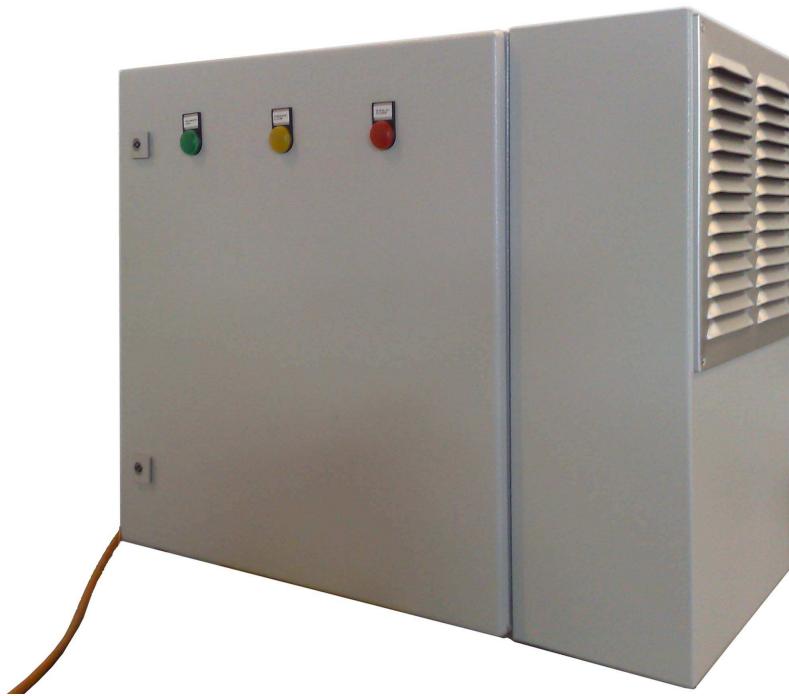
DR 16-46:41 ink. 31880

Kissenfläche / cushion area: **60,52m<sup>2</sup>**

12.08.2013	A	A. Janßen	new geometry
Date	Rev	Rev.- Name	Revision
	vector foiltec GmbH		Drawing
	Steinacker 3 · D-28717 Bremen		Übersicht - Kissen
	Telefon +49-(0)-421-69351-0		General Arrangement (GA) - cushions
	Telefax +49-(0)-421-69351-19		Kush 01-04
	de@vector-foiltec.com		
Drawn by	Date	Checked by	Project
A. Blohm	18.01.2013	XX	US - Billings MT - Parmly Library
File:			Drawing No Rev
			1059_9000 A
Remarks	Do not scale from this drawing - All dimensions in millimetres unless otherwise noted. All drawings are copyright and shall not be reproduced without the express consent of Vector Foiltec.		
	Print Date: <input type="text"/>		

## US - Data Sheet for Inflation Unit LK-350-115V IP65 For TEXLON® Systems

OPERATION AIR  
OUTLET Ø 3.93"

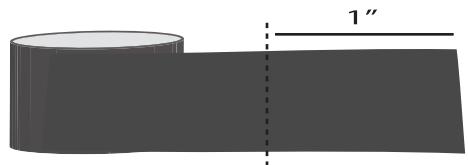


DIMENSIONS :	31.49" x 23.6" x 13.7"
LOCATION FOR INSTALLATION:	OUTSIDE
HOUSING:	STEEL SHEET 0,03"
POWDER COATED:	RAL 7035, LIGHT GREY
2 OPEN AREA AIR INTAKES:	RECTANGULAR, 11.6" x 11.6"
DIAMETER AIR OUTLET:	Ø 3.93"
VOLUME FLOW RATE.	APPROX. 175 CFM
WEIGHT:	APPROX. 145.5 LBS
POWER REQUIRED:	
POWER SYSTEM:	WYE POWER CONNECTION, SYSTEM VOLTAGE: 208VAC(208VAC BETWEEN PHASE AND PHASE AND 115VAC BETWEEN PHASE AND NEUTRAL)
CONNECTION CABLE:	5 WIRES WITH 3 CIRCUITS, N, PE
FUSES:	FOR EACH CIRCUIT USE SINGLE SLOW-BLOW FUSE WITH 25AMP EACH THE UNIT MAKES USE OF THE SECONDARY VOLTAGE (115VAC) ONLY.
POWER CONSUMPTION:	
NORMAL MODE:	185W
OPERATION UPSET ("BACK-UP"):	370W
OPTIONAL:	CONNECTION EXTERN (SIGNAL LAMP) OR BUILDING MANAGEMENT SYSTEM
NOISE DEVELOPMENT:	
NORMAL MODE:	APPROX. 56 DB(A)
OPERATION UPSET ("BACK-UP"):	APPROX. 59 DB(A)

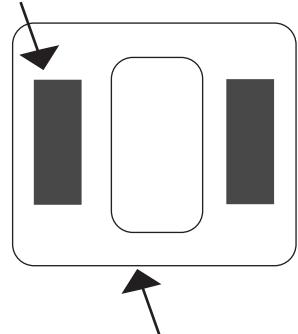
## Two Sided Eternabond Tape - Directions for use with MIRO Support

1. Clear an clean area of roof membrane.
2. Cut two 1" wide x 1" length strips.
3. Apply tape to two sides of the bottom of every MIRO pipe support.
4. Turn MIRO pipe support over and attach on desired area of roof membrane. Pipe support will not move after initial contact. Make sure to have desired location ready before attaching the MIRO pipe support.

\* Eternabond double sided tape is compatible with all roof membranes.  
\* MIRO will not be responsible for location or workmanship of attachment



Eternabond Double Sided Tape



Bottom View of MIRO Pipe Support

## Direcciones para utilizar Eternabond Tape de Doble Lado con el soporte fabricado por Industrias MIRO

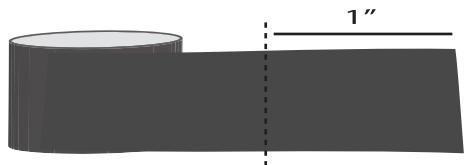
1. Limpie la area del techo donde va a posicionar los soportes.
2. Corte pedazos de tape que miden 1" x 1".
3. Aplique la cinta adhesiva a un lado debajo de cada tercer MIRO soporte.
4. Aplique el soporte MIRO en el puesto deseado. El soporte no se va a mover despues del inicio contacto con la superficie del techo. Asegurese que la area donde va a posicionar el soporte estece listo y limpio.

\* Eternabond tape es comparable con toda clase de techos.  
\* MIRO no mantiene responsabilidad sobre la mano de obra en la posicion del soporte, ni la colocacion del soporte.

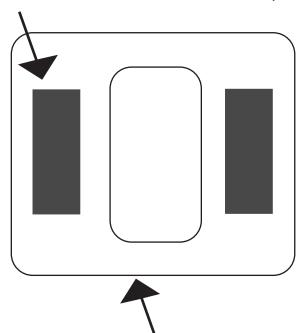
## Two Sided Eternabond Tape - Directions for use with MIRO Support

1. Clear an clean area of roof membrane.
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Eternabond Double Sided Tape



Bottom View of MIRO Pipe Support

## Direcciones para utilizar Eternabond Tape de Doble Lado con el soporte fabricado por Industrias MIRO

1. Limpie la area del techo donde va a posicionar los soportes.
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3. Aplique la cinta adhesiva a un lado debajo de cada tercer MIRO soporte.
4. Aplique el soporte MIRO en el puesto deseado. El soporte no se va a mover despues del inicio contacto con la superficie del techo. Asegurese que la area donde va a posicionar el soporte estece listo y limpio.

\* Eternabond tape es comparable con toda clase de techos.  
\* MIRO no mantiene responsabilidad sobre la mano de obra en la posicion del soporte, ni la colocacion del soporte.



PRODUCTS

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## Mechanical Unit Supports » Mechanical Unit Support-HD

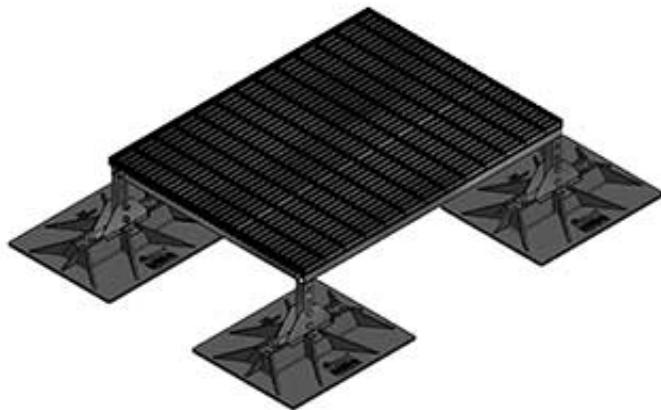
**Used to elevate rooftop mechanical units.**

**Heavy Duty Use.**



[Download Specs \(PDF\)](#)  
[Download Drawings \(PDF\)](#)  
[Download Drawings \(DWG\)](#)

**Product Description:** MIRO Mechanical Unit Supports are supports to elevate rooftop mechanical units such as air conditioning or other devices. The Mechanical Unit Supports are designed so that the mechanical units rest in heavy-duty corner or side supports which are connected to bases designed with MIRO technology to protect the roof membrane. The Mechanical Unit Supports are adjustable in that the strut can be selected to the appropriate height and there is provided an additional threaded rod which allows fine adjustment to level the rooftop units. All metal pieces are made out of hot-dip galvanized steel for outdoor weathering protection. The MIRO Mechanical Unit Support consists of (1) a MIRO designed base with gently curved edges to protect the roof membrane and to distribute the weight over the maximum roof surface, (2) hot-dip galvanized steel strut and all-thread height adjustable column.



## Mechanical Unit Support-HD

### Key Information » Mechanical Unit Support-HD

<b>Base Material:</b>	MIRON TPC or Polycarbonate Stainless Galvanized
<b>Size:</b>	Custom design required
<b>Max Load Weight:</b>	Maximum load weight is equivalent to and is part of the maximum roof top bearing load which MIRO has designed for its bases. MIRO recommends such loading not exceed two pounds per square inch.
<b>Spacing:</b>	The Mechanical Unit Support should be spaced at intervals so as to allow proper installation of the mechanical units or devices.

Patent numbers: 4502653; 4513934; 6364256; 6520456; D496058; D490295; D427049; D433615; D436522; D466393; D466394; D498133; D498660; D498661 and other patents pending

### More Mechanical Unit Supports from MIRO®

#### Mech Support-LD



MIRON TPC or  
Polycarbonate

#### Mech Support-HD



MIRON TPC or  
Polycarbonate  
Stainless  
Galvanized

**PRODUCTS »** Pipe Supports | Conduit Supports | Pipe Hangers

Duct or "H" Type and Cable Tray Supports | Mechanical Unit Supports

[Bridge Crossover & Ramp](#) | [Walkway & Service Platform](#)  
[Seismic & Wind Up-Lift](#) | [Accessories](#) | [UNI-FLEX](#)  
[Rooftop Sleeper Support](#)



**MIRO Industries, Inc.**

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PRODUCTS

COMPANY

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## Conduit Supports » Model 12-Base Strut-7 SS

**12" strut pipe support system used for ganging roof-mounted gas pipe.**

**Product Description:** A pipestand with a strut and clamp pipe support system used for ganging roof-mounted gas pipes, electrical conduit, solar piping and other mechanical piping. Unique design absorbs thermal expansion and contraction of pipes thus preventing damage to the roof membrane. Pipes rest on a 12" length of strut which is mounted on the base. The pipes can then be fastened by using typical clamps locked to the strut.



### Model 12-Base Strut-7 SS


[Download Specs \(PDF\)](#)
[Download Drawings \(PDF\)](#)

### Key Information » Model 12-Base Strut-7 SS

<b>Base Material:</b>	Stainless
<b>Size:</b>	The deck base is 12" by 16" , has a maximum bar length of 12".
<b>Max Pipe Clearance:</b>	Can adjust in height from a low of 2.5" to a high of 7.5" in elevation above the roof membrane.
<b>Max Load Weight:</b>	Maximum load weight may not exceed 150 lbs. per pipestand.
<b>Spacing:</b>	Manufacturer's recommended spacing is not to exceed 10 foot centers depending upon the load. Do not exceed 100 lbs. load weight.

Patent numbers: 4502653; 4513934; 6364256; 6520456; D496058; D490295; D427049; D433615; D436522; D466393; D466394; D498133; D498660; D498661 and other patents pending

### More Conduit Supports from Miro®

#### 2.5-CS-2



MIRON TPC™ or Polycarbonate

#### 2.5-CS-5



MIRON TPC™ or Polycarbonate

#### 2.5-CS-7



MIRON TPC™ or Polycarbonate

#### 2.5-CS-12



MIRON TPC™ or Polycarbonate

#### 12-Base Strut-7



Stainless Galvanized

#### 16-Base Strut-7


 MIRON TPC™ or Polycarbonate  
Stainless Galvanized

#### 16-Base Strut-12



MIRON TPC™ or Polycarbonate

#### 20-Base Strut-7



MIRON TPC™ or Polycarbonate

#### 20-Base Strut-12



MIRON TPC™ or Polycarbonate

#### 24-Base Strut-4



MIRON TPC™ or Polycarbonate

#### 24-Base Strut-18



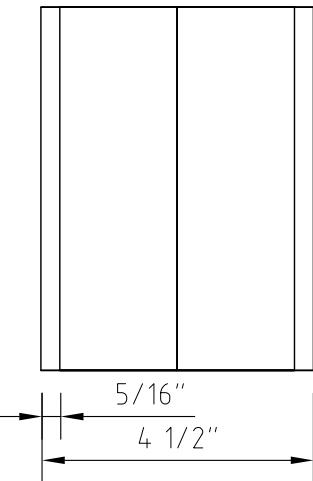
MIRON TPC™ or Polycarbonate

**PRODUCTS** » [Pipe Supports](#) | [Conduit Supports](#) | [Pipe Hangers](#)[Duct or "H" Type and Cable Tray Supports](#) | [Mechanical Unit Supports](#)[Bridge Crossover & Ramp](#) | [Walkway & Service Platform](#)[Seismic & Wind Up-Lift](#) | [Accessories](#) | [UNI-FLEX](#)[Rooftop Sleeper Support](#)**MIRO Industries, Inc.**

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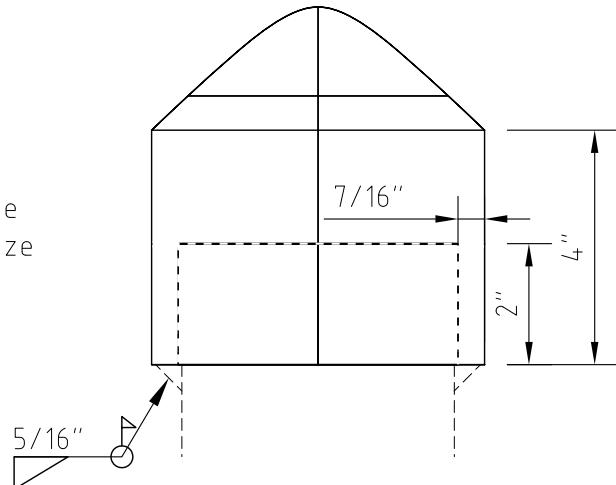
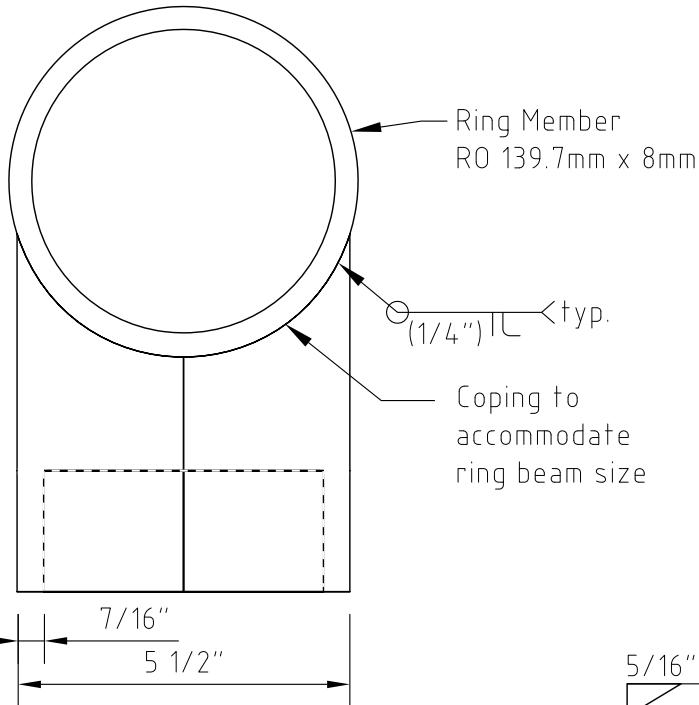
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001  
Coping Axon View

002  
Post Side View



002  
Coping Side views

A36 Steel tube D.O.M.  
5-1/2" OD

Date	Rev	Rev.- Name	Revision	Project status	
	vector foiltec	13 Green Mountain Drive Cohoes, NY 12047 Phone: 518-783-0575 Fax: 518-783-0474 us@vector-foiltec.com	Drawing <b>Ring Beam Connection Details</b>	<b>PRELIMINARY</b>	
Drawn by kh	Date 10/11/13	Checked by cm	Project <b>US-MT-Billings-Parmly Library</b>	Scale/ Unit NTS	Size letter
				Drawing No beam con.	Rev

File: B:\05-Project Files\01-active projects\1059-US-MT-Parmly Billings Library Skylight\09 Drawing Source Files\0000-Sketches\SK\_2013.10.11 Ring Beam Connection.dwg 10/11/2013 2:57:42 PM

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UNITED STATES PARMLY BILLINGS LIBRARY SKYLIGHT  
BILLINGS, MONTANA

---

**Weld Calculations for Structural Steel Ring Supports**

DATE OF ISSUE: *November 19, 2013*

PREPARED FOR: Vector Foiltec  
13 Green Mountain Drive  
Cohoes, NY 12047



Weidlinger Associates, Inc. • 27 Wormwood Street, 2<sup>nd</sup> Floor • Boston, MA 02210  
Phone 617 374 0000 • Fax 617 374 0010 • [www.wai.com](http://www.wai.com)



PROJECT	PARMLY LIBRARY	
BY	JC	DATE 10-24-2013
CHECKED BY		DATE

SUBJECT		
PAGE	1	OF

A. PROVIDE WELD DESIGN FOR POST CONNECTION

$$\text{TUBE STRENGTH} = A360 \quad F_y = 360 \text{ ksi} \\ F_u = 58 \text{ ksi}$$

$$A_w = \pi d = 17.3 \text{ in}^2$$

$$\text{WELD STRENGTH} = .6 F_{exx} \\ = .6 (70) = 42 \text{ ksi}$$

BASE STRENGTH OF WELD ON STRENGTH  
OF SMALL TUBE

$$F_y A_g = (360 \text{ ksi}) (4.14 \text{ in}^2) = 149 \text{ kips}$$

$$\phi F_y A_g = 134 \text{ kips}$$

$$\phi F_u A_e = .75(58)(.6)(4.14) = \underline{\underline{108 \text{ kips}} \leftarrow}$$

$$\phi R_{nw} = \phi .6 F_{exx} (.707) D_{16} l_w \\ = .75(.6)(70)(.707) 5_{16} (17.3) = 120.4 \text{ kips} \leftarrow$$

USE  $5_{16}$ " FILLET WELD

NOTE MAX ULTIMATE REACTION PER CALC'S BY GRID ENGINEERS

$$F_x = 8.4 \text{ kips}$$

$$A_w = \pi d = 14.13 \text{ in}^2$$

$$F_y = .65 \text{ kips}$$

$$S_w = \pi d^2 / 4 = 15.9 \text{ in}^3$$

$$F_z = 37 \text{ kips}$$

$$M = 8.4(4") + .65(4") = 36.2 \text{ k-in}$$

$$f_R = \sqrt{(2.62^2 + 2.27^2) + (.64)^2} - 4.93 \text{ k-in} \\ f_p = 6.96 \text{ k/in}$$



PROJECT: PARMLY LIBRARY  
BY: JC DATE: 10-24-2013  
CHECKED BY: DATE:

SUBJECT:

PAGE 2 OF

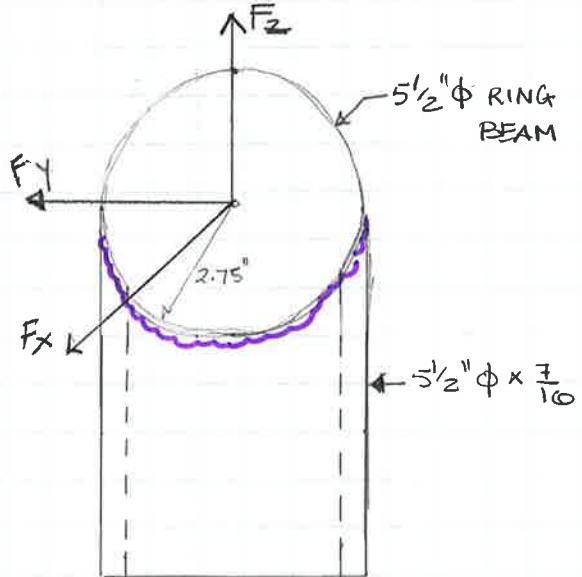
B. PROVIDE WELD BETWEEN RING AND SUPPORT

PERIMETER RING  
ROUND  $5\frac{1}{2}'' \phi \times \frac{5}{16}''$  WALL

POST  
 $5\frac{1}{2}'' \phi \times \frac{7}{16}''$

BASE DESIGN OF WELD FROM STRENGTH  
OF PERIMETER RING

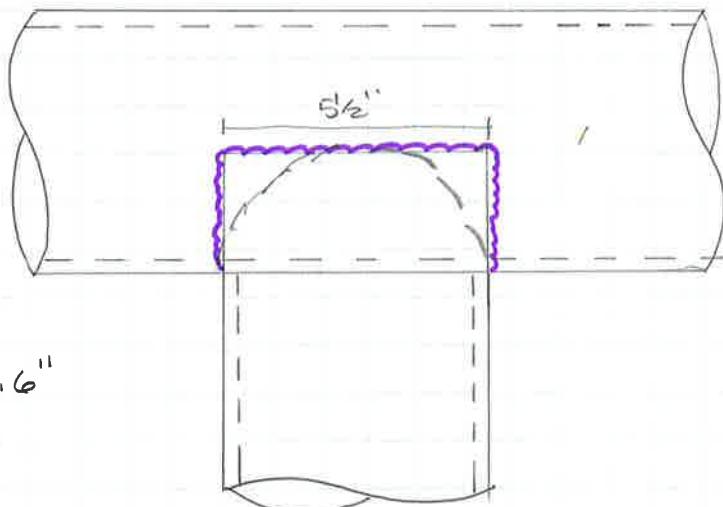
$$\phi R_{nw} = \phi \cdot 0.6 F_u A$$
$$= .75 (.6) (58)$$



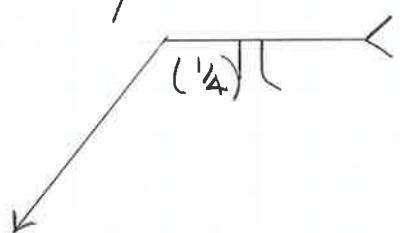
GROVE WELD (PJP)

$$\phi R_{nw} = \phi t_e \cdot 0.6 F_{exx} \times l_w$$
$$= .9 (.25) (.6) (70) (19.6)$$
$$= 185 \text{ kips}$$

$$l_w = 5\frac{1}{2} \times 2 + \frac{\pi (2.75)}{2} \times 2 \text{ SIDES} = 19.6''$$



USE PJP GROOVE WELD W/ EFFECTIVE  
THROAT =  $\frac{1}{4}''$



## APPENDIX

**VECTOR FOILTEC CALCULATIONS [NOT BY  
WEIDLINGER ASSOCIATES, INC.]  
(USED FOR REFERENCE ONLY)**

## Structural Calculation Texlon Cushions & Steel Structure

Project: USA - Parmly Billings Library - Billings MT

Subject: **Texlon Skylight  
ETFE, Extrusion and Steel Joints  
- Revision with new structure -**

Volume of this structural calculation: pages 1 to 26 and annexes A, B, C

D-Bremen, August 14, 2013



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## Introduction and Basics

### Item

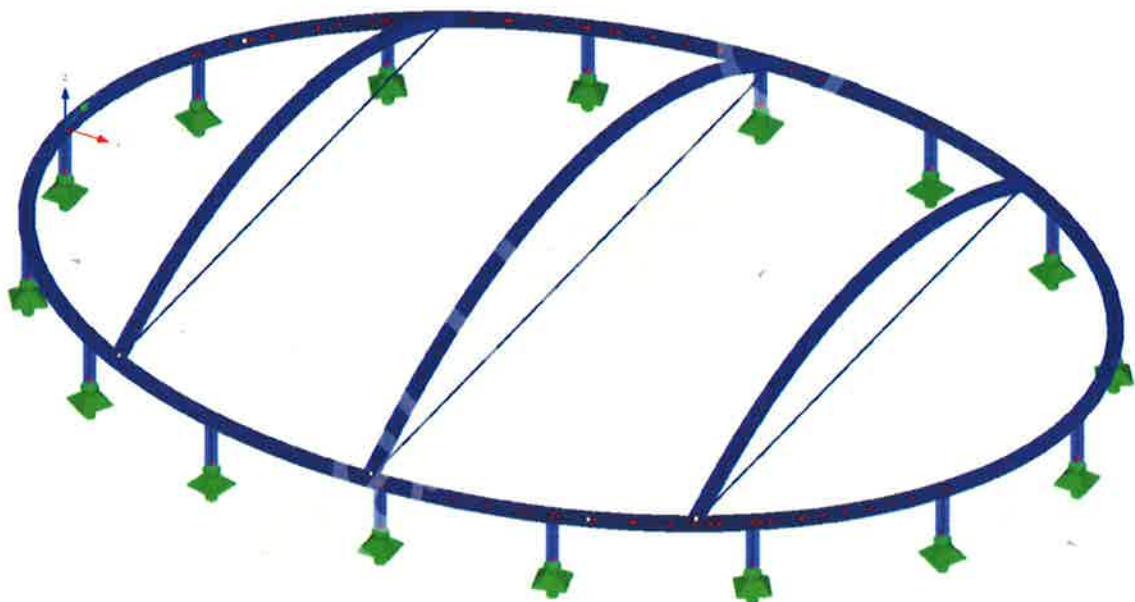
A skylight, made of ETFE cushion membran structure and structural steel, is subject of this structural analysis. The membran cushions and structural steel members are statically proved. Elements of primary structure are not subject and have to be designed by someone else.

Dimensions of the oval plan view are

$$a = 32 * 0,3048 = 9,754 \text{ m}$$

$$b = 24 * 0,3048 = 7,315 \text{ m}$$

$$\text{Area} = \pi * a * \frac{b}{4} = 56,0 \text{ m}^2$$



### Parts of the structural calculation

A new structural concept is analysed in this revision.

It substitutes all former parts from Dec. 2012/Jan. 2013

Part 1 - Structural steel design

Part 2 - ETFE membrane

Part 3 - Extrusion and steel joints

### Revisions

Rev.	Date	Remarks
0	10-08-2013	Structural system switched to 3 parallel Arches

**Basics**

[1] construction documents of May 01,2012:  
a0.4 - code/occupancy plans  
s2.2 - level 2 framing plan  
s2.3 - roof framing plan  
s6.1 - roof framing details  
a7.9 - exterior details  
by will bruder + Partners LTD, 2524 North 24. th Street, Phoenix, Arizona 85008, USA

[2] drawing 1059\_3000: General Arrangement, Steel Structure, Positioning of ring beam + columns  
(November 16, 2012) vector foiltec GmbH, Steinacker 3, D-28717 Bremen  
(vector foiltec llc, 13 Green Mountain Drive, Cohoes, NY 12047, USA)

[3] ASCE Standard 7-05: Minimum Design Loads for Buildings and other structures

[4] EN 1993-1-1 (2005): Design of steel structures - Part 1-1: General rules and rules for buildings

[5] EN 1993-1-8 (2010): Design of steel structures - Part 1-8: Design of joints

[6] EN 1990-0 (2002): Basis of structural design

conversion parameters

$$\begin{aligned} \text{onePSF} &= 0,04788 \text{ kN/m}^2 \\ \text{one_ksi} &= 6,895 \text{ N/mm}^2 \end{aligned}$$

**Material**

structural steel members

Grade S355 J2H

$$\text{youngs modulus } E = 210000 \text{ N/mm}^2$$

$$E_{US} = E / 6,895 = 30456,9 \text{ ksi}$$

$$\text{yield strength } f_y = 355 \text{ N/mm}^2$$

$$f_{y,US} = f_y / 6,895 = 51,49 \text{ ksi}$$

$$\text{ultimate strength } f_u = 510 \text{ N/mm}^2$$

$$f_{u,US} = f_u / 6,895 = 73,97 \text{ ksi}$$

if not mentioned otherwise

## Notes on Structural Analyses (Annex A - C)

### Annex A

ETFE stress check under downward loads

Loads: Self weight + snow load 30 psf

$$\begin{aligned} \text{net clearance ... e} &= 0,30 \text{ m} \\ \text{uniform load... s} &= 30 * \text{onePSF} = 1,436 \text{ kN/m}^2 \\ \text{element load... se} &= s * e/2 = 0,215 \text{ kN/m} \end{aligned}$$

Design:

upper layer  $\geq 200 \mu\text{m}$ , initial camber 6%  
 middle layer  $\geq 100 \mu\text{m}$   
 bottom layer  $\geq 200 \mu\text{m}$ , initial camber 6%

Stress Check:

$$\begin{aligned} \text{received internal stress ... } \sigma &= 12,15 \text{ N/mm}^2 \\ \text{allowable stress... } \sigma_{\text{allow}} &= 12,00 \text{ N/mm}^2 \\ \text{degree of utilisation... } \eta &= \sigma / \sigma_{\text{allow}} = 1,01 \leq 1 \end{aligned}$$

### Annex B

ETFE and arched steel girder stress check under lifting loads

Loads: Self weight + wind load load 20,4 psf

$$\begin{aligned} \text{net clearance ... e} &= 0,30 \text{ m} \\ \text{uniform load... w} &= 20,4 * \text{onePSF} = 0,977 \text{ kN/m}^2 \\ \text{element load... we} &= w * e/2 = 0,147 \text{ kN/m} \end{aligned}$$

Design:

upper layer  $\geq 200 \mu\text{m}$ , initial camber 6%

Stress Check (stresses received from an analysis with 3% initial camber):

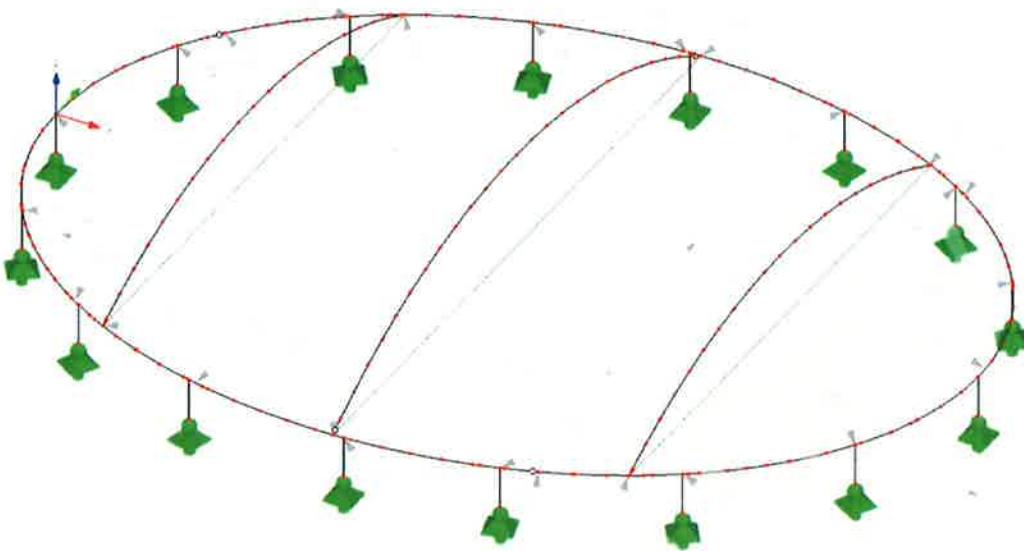
$$\begin{aligned} \text{received internal stress ... } \sigma &= 17,80 \text{ N/mm}^2 \\ \text{allowable stress... } \sigma_{\text{allow}} &= 18,00 \text{ N/mm}^2 \\ \text{degree of utilisation... } \eta &= \sigma / \sigma_{\text{allow}} = 0,99 \leq 1 \end{aligned}$$

### Annex C

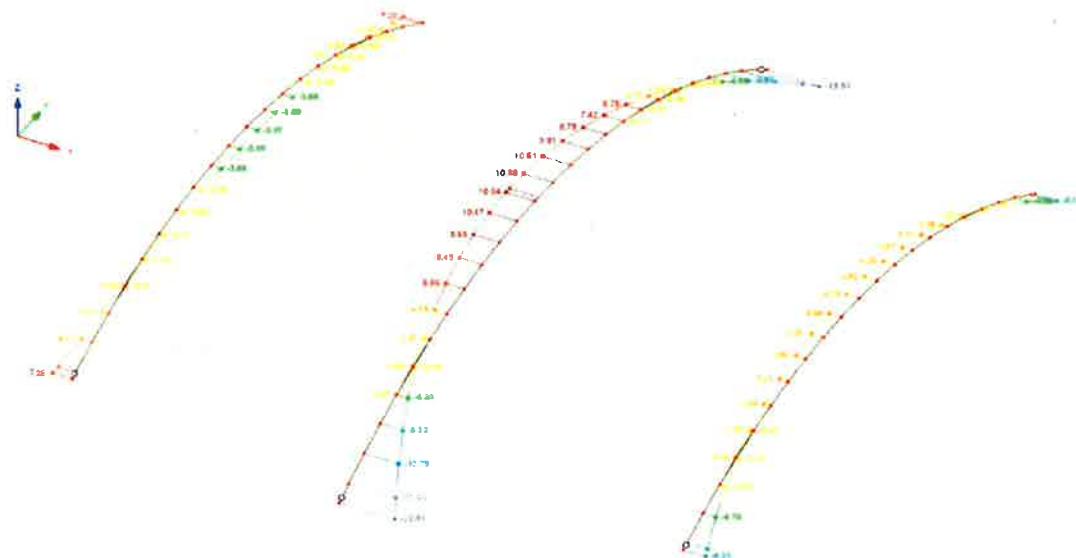
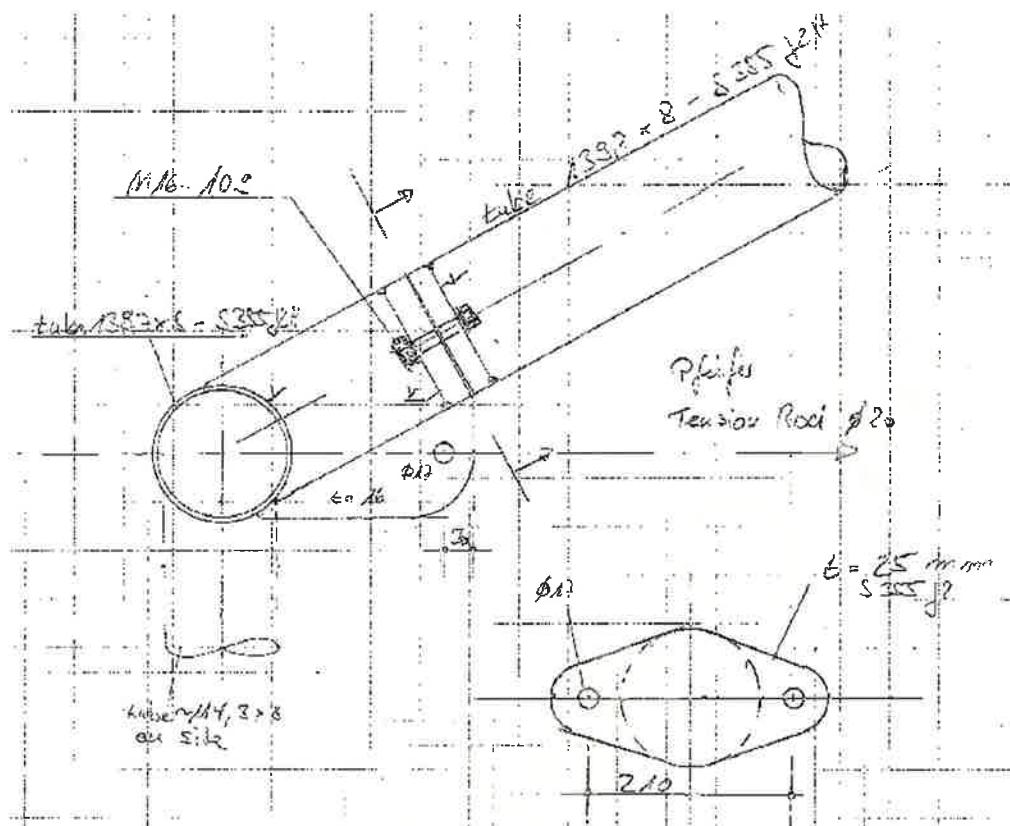
Steel structure stress check under downward loads

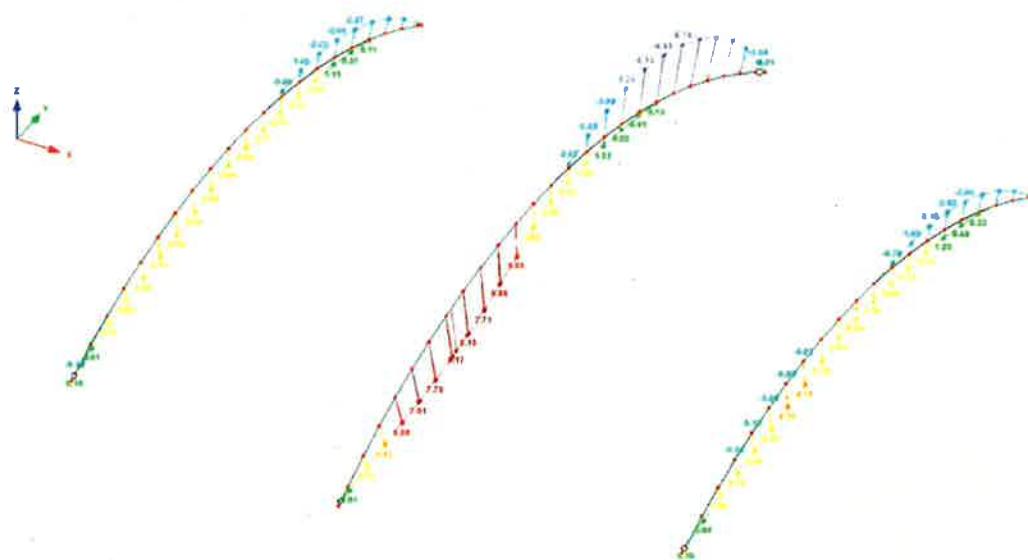
■ SPANNUNGEN QUERSCHNITTSWEISE									
Quer.-Nr.	Stab-Nr.	x-Stelle [mm]	S-Punkt-Nr.	Last-fall	Spannungsart	Spannung vorh	[N/mm <sup>2</sup> ] grenz	Aus-nutzung	
2	<b>RO 114.3x8 - Column</b>			LG11	Sigma gesamt	-115.523	218.182	0.53	
	495	451.9	26		Tau gesamt	12.053	125.967	0.10	
	495	0.0	17		Sigma-v	115.527	218.182	0.53	
5	<b>RO 139.7x8 - Ringbeam</b>			LG13	Sigma gesamt	-113.618	327.273	0.35	
	789	0.0	4		Tau gesamt	25.858	188.951	0.14	
	1118	70.2	21		Sigma-v	114.706	327.273	0.35	
6	<b>RO 139.7x8 - Archbeams</b>			LG13	Sigma gesamt	-214.734	327.273	0.66	
	815	0.0	19		Tau gesamt	-17.908	188.951	0.09	
	809	0.0	11		Sigma-v	214.798	327.273	0.66	
7	<b>Rundstahl 20 - Rod</b>			LG11	Sigma gesamt	172.190	327.273	0.53	
	817	7200.9	37		Tau gesamt	0.000	188.951	0.00	
	296	0.0	1		Sigma-v	172.190	327.273	0.53	

## Steel Joints and Aluminium Extrusion



## Pos. 31 T-Joint Arches to Eaves Ring





### Rating Forces

Min  $N_d$  = - 68 kN  
 $V_{yd}$  = 49,0 kN  
 $V_{zd}$  = 2,40 kN  
 $M_{yd}$  = 3,1 kNm  
 $M_{zd}$  = 0,0 kNm  
 $M_{z0}$  = 2,81 kNm

### schrauben (Bolted joints)

$$V_{zd1} = \frac{310}{24} \cdot \frac{1+0,24}{2} = 16,62 \text{ kN}$$

$$V_{yd1} = 13,0 / 2 = 6,5 \text{ kN}$$

$$V_{d1} = \sqrt{16,62^2 + 6,5^2} = 18,2 \text{ kN}$$

$$F_t = 2021 / 21 = 93,1 \text{ kN}$$

gew M16 - 10,9

$$F_t / F_{t,Rd} = \frac{93,1}{113} = 0,83 < 1$$

$$V_{d1} / V_{d,Rd} = \frac{18,2}{62,8} = 0,3 < 1$$

$$0,3 + \frac{0,83}{113} = 0,33 < 1$$

stirnplatte (End Plate)

$$M = 39,11 \times 4,0 = 39,64 \text{ kNm}$$

$$w = 11 \text{ cm}$$

$$E = 2,5$$

$$W = M \cdot 2,5^2 / 4 = 17,2$$

$$387 / 17,2 = 23,17 \text{ kN/cm}^2$$

$$237 / 35,5 = 0,65 < 1$$

**Welded tube joint**

(according to DIN EN 1993-1-8: 2010-12)

**Einwirkungen (Acting Forces and Bending Moments)****Strebe (Brace)** Column Member No. 327

$$N_{1,Ed} = -68,00 \text{ kN}$$

$$\text{in-plane } M_{ip,1,Ed} = 20,81 \text{ kNm}$$

$$\text{out-of-plane } M_{op,1,Ed} = 0,00 \text{ kNm}$$

**Gurtstab (Chord)** Eaves Ring Member No. 292

$$N_{0,Ed} = -14,70 \text{ kN}$$

$$\text{pressure } > 0 \dots N_{p,Ed} = -N_{0,Ed} = 14,70 \text{ kN}$$

$$\text{in-plane } M_{ip,0,Ed} = 11,20 \text{ kNm}$$

$$\text{out-of-plane } M_{op,0,Ed} = 9,00 \text{ kNm}$$

**Geometrie (geometry)**

$$a_w = t_1$$

$$\Theta_1 = 90,00^\circ$$

$$\text{partial safety factor according to Eurocode } \gamma_{M5} = 1,00$$

**Strebe (brace)**

$$f_{y1} = f_y/10 = 35,50 \text{ kN/cm}^2$$

$$d_1 = 139,7 / 10 = 13,97 \text{ cm}$$

$$t_1 = 8,0 / 10 = 0,80 \text{ cm}$$

**Gurtstab (chord)**

$$f_{y0} = f_y/10 = 35,50 \text{ kN/cm}^2$$

$$d_0 = 139,7 / 10 = 13,97 \text{ cm}$$

$$t_0 = 8 / 10 = 0,80 \text{ cm}$$

$$A_0 = \pi/4 * (d_0^2 - (d_0 - 2*t_0)^2) = 33,10 \text{ cm}^2$$

$$W_{el,0} = \pi/(32*d_0) * (d_0^4 - (d_0 - 2*t_0)^4) = 103,12 \text{ cm}^3$$

**Gurtspannung (Chord Stress)** pressure > 0, tension < 0

$$\sigma_{p,Ed} = \frac{N_{p,Ed} + \sqrt{(M_{ip,0,Ed})^2 + (M_{op,0,Ed})^2} \times 10^{-2}}{A_0 + W_{el,0}} = 14,38 \text{ kN/cm}^2$$

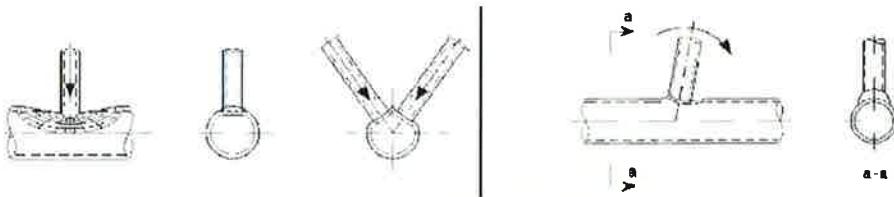
**Parameter und Beiwerte (Coefficients):**

$$\begin{aligned}\beta &= d_1 / d_0 &= 1,00 \\ \gamma &= d_0 / (2 * t_0) &= 8,73 \\ \text{pressure } > 0! \dots n_p &= \sigma_{p,Ed} / f_{y0} * \gamma_{M5} &= 0,405 \\ k_p &= \text{MIN}(1-0,3 * n_p * (1+n_p); 1,0) &= 0,829 \\ k_p &= \text{WENN}(n_p < 0; 1,0; k_p) &= 0,829\end{aligned}$$

**Gültigkeitsbereich nach Tab. 7.1 (Scope according to Eurocode table 7.1):**

$$\begin{aligned}\text{Durchmesserverhältnis} &= d_1 / d_0 &= 1,0 & (\leq 1,0 \text{ und } \geq 0,2) \\ \text{(diameter ratio)} \\ \text{Gurtstäbe Zug} &= d_0 / t_0 &= 17,5 & (\leq 50 \text{ und } \geq 10) \\ \text{(chord in tension)} \\ \text{Gurtstäbe Druck} &= d_0 / t_0 &= 17,5 & (\leq 50 \text{ und } \geq 10 \text{ sowie Klasse 1 oder 2}) \\ \text{(chord in pressure, needs to be class 1 or 2 section} &\Rightarrow \text{plastic design possible}) \\ \text{Streben Zug} &= d_1 / t_1 &= 17,5 & (\leq 50) \\ \text{(brace in tension)} \\ \text{Streben Druck (brace in pressure)} & & & \text{Klasse 1 oder 2 (class 1 or 2)}\end{aligned}$$

**Flanschversagen des Gurtstabes (flange failure of the chord)**



$$N_{1,Rd,a} = \frac{\gamma^{0,2} * k_p * f_{y0} * t_0^2}{\sin(\Theta_1)} * (2,8 + 14,2 * \beta^2) / \gamma_{M5} = 494 \text{ kN}$$

$$M_{ip,1,Rd,a} = \frac{f_{y0} * t_0^2 * d_1}{\sin(\Theta_1)} * \sqrt{\gamma} * \beta * k_p / \gamma_{M5} * 10^{-2} = 37,71 \text{ kNm}$$

$$M_{op,1,Rd,a} = \frac{f_{y0} * t_0^2 * d_1}{\sin(\Theta_1)} * \frac{2,7}{1 - 0,81 * \beta} * k_p / \gamma_{M5} * 10^{-2} = 37,39 \text{ kNm}$$

**Durchstanzen der Wandung des Gurtstabes (punching shear)**

nur bei (only to be considered if)  $d_1 \leq d_0 - 2 * t_0$

$$\eta = d_1 / (d_0 - 2 * t_0) = 1,13 \leq 1$$



$$N_{1,Rd,d} = \text{WENN}(\eta \leq 1; \frac{f_{y0}}{\sqrt{3}} * t_0 * \pi * d_1 * \frac{1 + \sin(\Theta_1)}{2 * (\sin(\Theta_1))^2} * \frac{1}{\gamma_{M5}}; N_{1,Rd,a}) = 494,00 \text{ kN}$$

$$M_{ip,1,Rd,d} = \text{WENN}(\eta \leq 1; \frac{f_{y0} * t_0 * d_1^2}{\sqrt{3}} * \frac{1 + 3 * \sin(\Theta_1)}{4 * (\sin(\Theta_1))^2} / \gamma_{M5} * 10^{-2}; M_{ip,1,Rd,a}) = 37,71 \text{ kNm}$$

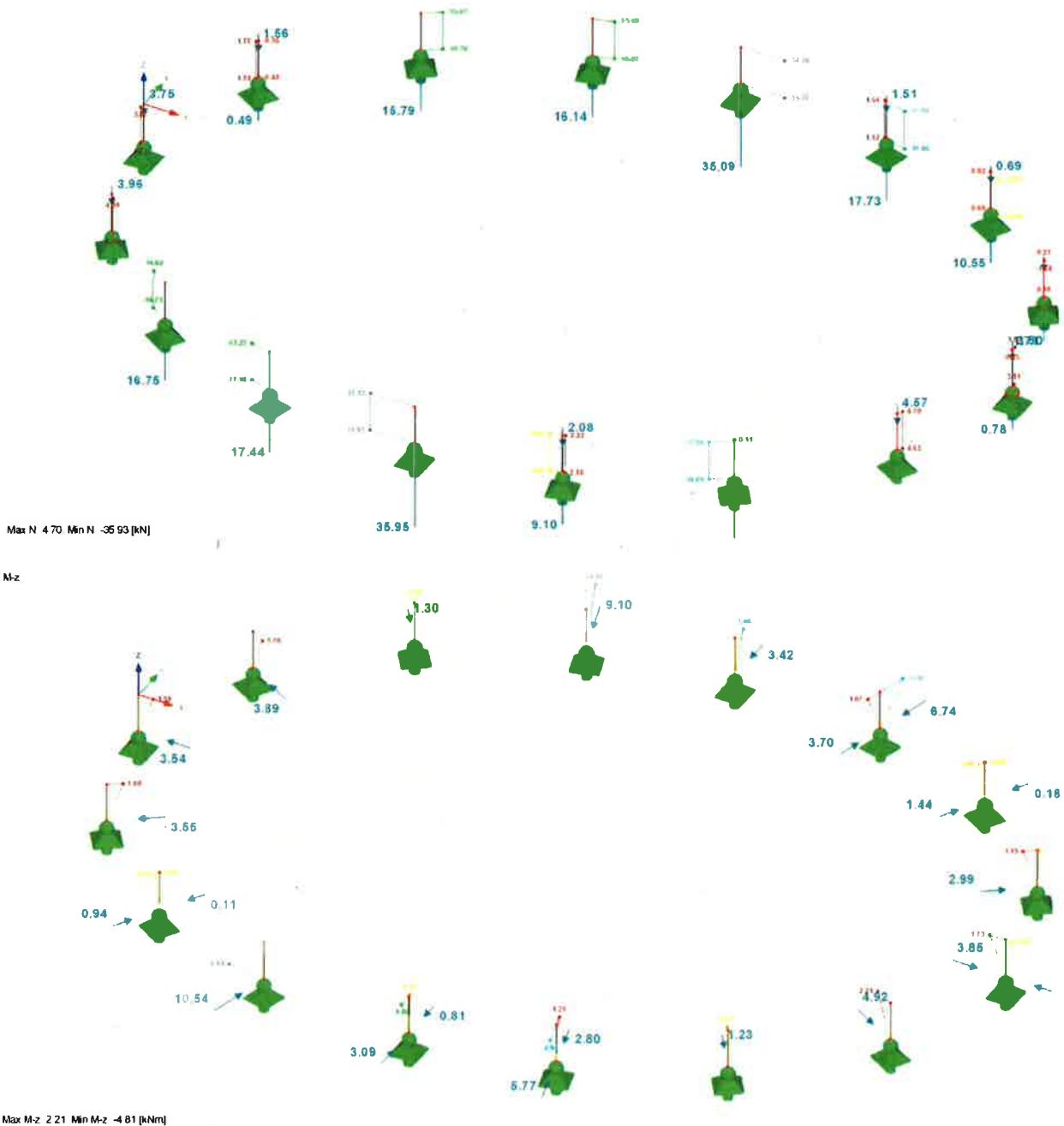
$$M_{op,1,Rd,d} = \text{WENN}(\eta \leq 1; f_{y0} * t_0 * \frac{d_1^2}{\sqrt{3}} * \frac{3 + \sin(\Theta_1)}{4 * (\sin(\Theta_1))^2} * \frac{1}{\gamma_{M5} * 10^{-2}}; M_{op,1,Rd,a}) = 37,39 \text{ kNm}$$

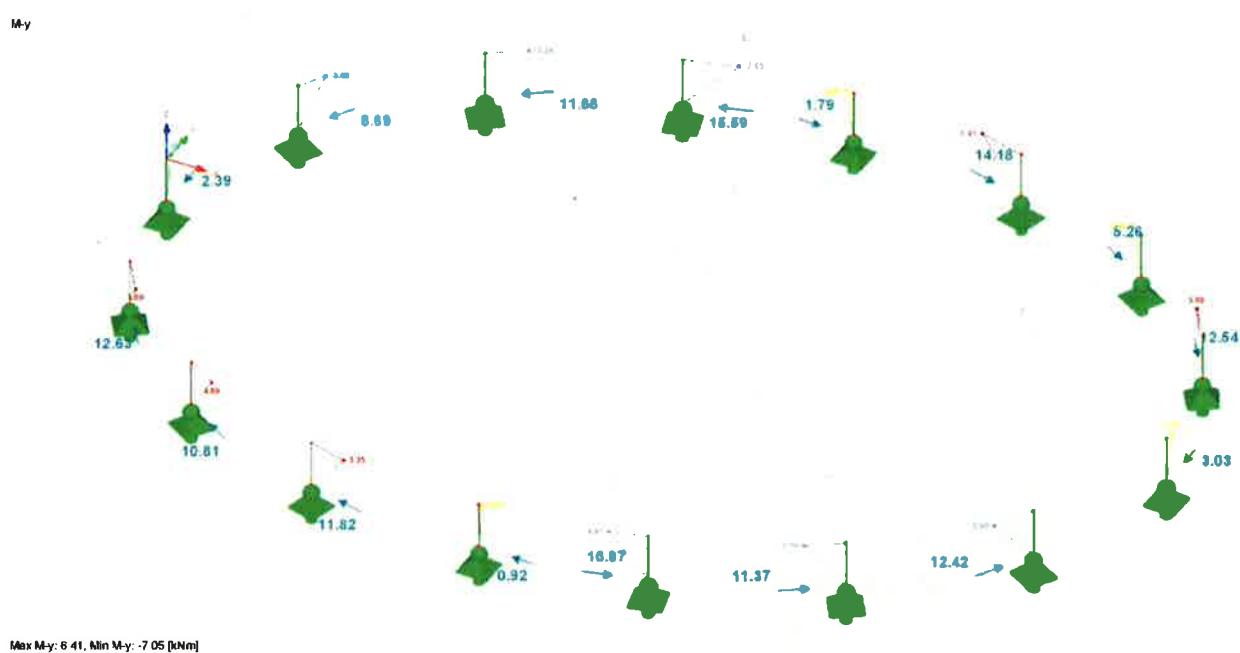
**Nachweis (Degree of Utilisation)**

Normalkraft $\eta_N$ =	$ABS(N_{1,Ed})/MIN(N_{1,Rd,a};N_{1,Rd,d})$	= <u>0,14 &lt; 1,0</u>
Moment in-plane $\eta_{ip}$ =	$M_{ip,1,Ed}/MIN(M_{ip,1,Rd,a};M_{ip,1,Rd,d})$	= <u>0,55 &lt; 1,0</u>
Moment out-plane $\eta_{op}$ =	$M_{op,1,Ed}/MIN(M_{op,1,Rd,a};M_{op,1,Rd,d})$	= <u>0,00 &lt; 1</u>
Interaktion $\eta$ =	$\eta_N + \eta_{ip}^2 + \eta_{op}$	= <u>0,44 &lt; 1</u>

## Pos. 32 T-Joint Column to Eaves Ring

(according to DIN EN 1993-1-8: 2010-12)





### Einwirkungen (Acting Forces and Bending Moments)

**Strebe (Brace)** internal forces worst of column members no. 329, 332

$$N_{1,Ed} = -36,00 \text{ kN}$$

$$\text{in-plane } M_{ip,1,Ed} = 7,05 \text{ kNm}$$

$$\text{out-of-plane } M_{op,1,Ed} = 4,81 \text{ kNm}$$

**Gurtstab (Chord)** eaves ring members no. 166 and 13

$$N_{0,Ed} = -16,30 \text{ kN}$$

$$\text{pressure } > 0 \dots N_{p,Ed} = -N_{0,Ed} = 16,30 \text{ kN}$$

$$\text{in-plane } M_{ip,0,Ed} = 8,70 \text{ kNm}$$

out-of-plane  $M_{op,0,Ed} = 10,19 \text{ kNm}$

**Geometrie (Geometry)**

$a_w = t_1$

$\Theta_1 = 90,00^\circ$

$\gamma_{M5} = 1,00$

**Strebe (Brace)**

$f_{y1} = 241/10 = 24,10 \text{ kN/cm}^2$

$d_1 = 114,3 / 10 = 11,43 \text{ cm}$

$t_1 = 17,2 / 10 = 1,72 \text{ cm}$

**Gurtstab (Chord)**

$f_{y0} = f_y/10 = 35,50 \text{ kN/cm}^2$

$d_0 = 139,7 / 10 = 13,97 \text{ cm}$

$t_0 = 8 / 10 = 0,80 \text{ cm}$

$A_0 = \pi/4 * (d_0^2 - (d_0 - 2 * t_0)^2) = 33,10 \text{ cm}^2$

$W_{el,0} = \pi/(32 * d_0) * (d_0^4 - (d_0 - 2 * t_0)^4) = 103,12 \text{ cm}^3$

**Gurtspannung (Stress in chord, pressure with pos. algebraic sign):**

$$\sigma_{p,Ed} = \frac{N_{p,Ed}}{A_0} + \frac{\sqrt{(M_{ip,0,Ed})^2 + (M_{op,0,Ed})^2} \times 10^2}{W_{el,0}} = 13,49 \text{ kN/cm}^2$$

**Parameter und Beiwerke (Coefficients):**

$\beta = d_1 / d_0 = 0,82$

$\gamma = d_0 / (2 * t_0) = 8,73$

pressure >0!...  $n_p = \sigma_{p,Ed} / f_{y0} * \gamma_{M5} = 0,380$

$k_p = \text{MIN}(1-0,3 * n_p * (1+n_p); 1,0) = 0,843$

$k_p = \text{WENN}(n_p < 0; 1,0; k_p) = 0,843$

**Gültigkeitsbereich nach Tab. 7.1 (Scope according to Eurocode table 7.1):**

Durchmesserverhältnis =  $d_1 / d_0 = 0,8 (\leq 1,0 \text{ und } \geq 0,2)$

(diameter ratio)

Gurtstäbe Zug =  $d_0 / t_0 = 17,5 (\leq 50 \text{ und } \geq 10)$

(chord in tension)

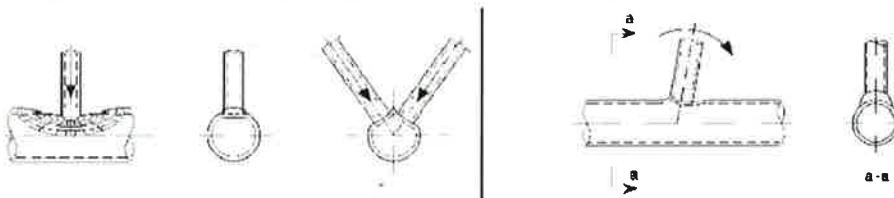
Gurtstäbe Druck =  $d_0 / t_0 = 17,5 (\leq 50 \text{ und } \geq 10 \text{ sowie Klasse 1 oder 2})$

(chord in pressure, needs to be class 1 or 2 section  $\Rightarrow$  plastic design possible)

Streben Zug =  $d_1 / t_1 = 6,6 (\leq 50)$

(brace in tension)

Streben Druck (brace in pressure) **Klasse 1 oder 2 (class 1 or 2)**

**Flanschversagen des Gurtstabes (Flange Failure of the Chord)**

$$N_{1,Rd,a} = \frac{\gamma^{0,2} * k_p * f_{y0} * t_0^2}{\sin(\Theta_1)} * (2,8 + 14,2 * \beta^2) / \gamma_{M5} = 365 \text{ kN}$$

$$M_{ip,1,Rd,a} = 4,85 * \frac{f_{y0} * t_0^2 * d_1}{\sin(\Theta_1)} * \sqrt{\gamma} * \beta * k_p / \gamma_{M5} * 10^{-2} = 25,72 \text{ kNm}$$

$$M_{op,1,Rd,a} = \frac{f_{y0} * t_0^2 * d_1}{\sin(\Theta_1)} * \frac{2,7}{1 - 0,81 * \beta} * k_p / \gamma_{M5} * 10^{-2} = 17,60 \text{ kNm}$$

### Durchstanzen der Wandung des Gurtstabes (Punching Shear)

only to be considered if  $d_1 \leq d_0 - 2 * t_0$

$$\eta = d_1 / (d_0 - 2 * t_0) = 0,92 \leq 1$$



$$N_{1,Rd,d} = \text{WENN}(\eta \leq 1; \frac{f_{y0}}{\sqrt{3}} * t_0 * \pi * d_1 * \frac{1 + \sin(\Theta_1)}{2 * (\sin(\Theta_1))^2} * \frac{1}{\gamma_{M5}}; 0) = 588,78 \text{ kN}$$

$$M_{ip,1,Rd,d} = \text{WENN}(\eta \leq 1; \frac{f_{y0} * t_0 * d_1^2}{\sqrt{3}} * \frac{1 + 3 * \sin(\Theta_1)}{4 * (\sin(\Theta_1))^2} / \gamma_{M5} * 10^{-2}; 0) = 21,42 \text{ kNm}$$

$$M_{op,1,Rd,d} = \text{WENN}(\eta \leq 1; f_{y0} * t_0 * \frac{d_1^2}{\sqrt{3}} * \frac{3 + \sin(\Theta_1)}{4 * (\sin(\Theta_1))^2} * \frac{1}{\gamma_{M5} * 10^{-2}}; 0) = 21,42 \text{ kNm}$$

### Nachweis (Degree of Utilisation)

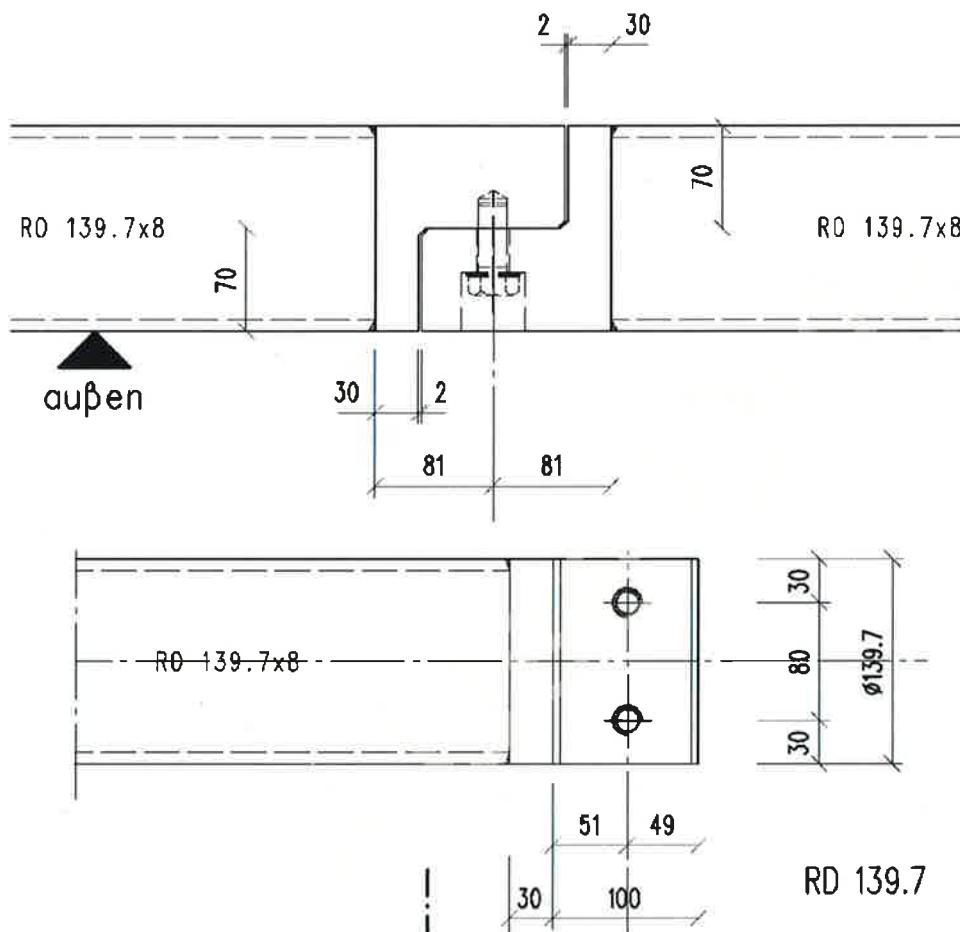
$$\text{Normal Force } \eta_N = \text{ABS}(N_{1,Ed}) / \text{MIN}(N_{1,Rd,a}; N_{1,Rd,d}) = 0,10 < 1,0$$

$$\text{Moment in-plane } \eta_{ip} = M_{ip,1,Ed} / \text{MIN}(M_{ip,1,Rd,a}; M_{ip,1,Rd,d}) = 0,33 < 1,0$$

$$\text{Moment out-plane } \eta_{op} = M_{op,1,Ed} / \text{MIN}(M_{op,1,Rd,a}; M_{op,1,Rd,d}) = 0,27 < 1,0$$

$$\text{Interaction } \eta = \eta_N + \eta_{ip}^2 + \eta_{op} = 0,48 < 1$$

### Pos. 33 Bolted Joints within the Eaves Ring



#### Geometry

section diameter...  $d_0$  = 139,7 mm  
 section wall thickness...  $t_0$  = 8,0 mm  
 joint plate thickness...  $t_p$  = 30,0 mm  
 ultimate strength..  $f_u$  = 510 N/mm<sup>2</sup>  
 bolt diameter...  $d_b$  = 20,0 mm  
 grade 10.9...  $f_{ub}$  = 1000 N/mm<sup>2</sup>  
 design tension resistance...  $FtRd$  = 176,0 kN  
 design shear resistance...  $FvRd$  = 98,0 kN  
 (tread in the shear surface!)

gap between the bolts...  $e_b$  = 80,0 mm  
 design punsch resistance  $BpRd$  =  $248 * t_p/10 = 744,0$  kN  
 (248 kN acc. to a table for M20-10.9, 10 mm plate -S355)  
 bolt hole diameter...  $d_h$  =  $d_b + 2 = 22$  mm

partial material safety factor... $\gamma M0$ =	1,00
partial connection safety factor... $\gamma M2$ =	1,25

### Acting Internal Forces

LG3, eaves ring members no. 309 + 312, see annex A

pressure force...  $NEd$  = 16,30 kN

horizontal shear force...  $VyEd$  = 5,26 kN

vertical shear force...  $VzEd$  = 10,87 kN

torsional moment...  $TEd$  = 3,14 kNm

forces acting on one bolt

$$FvEd = \sqrt{(VzEd^2 + NEd^2)/2} = 9,8 \text{ kN}$$

$$FtEd = 0,5*VyEd + 1000*TEd / eb = 41,9 \text{ kN}$$

### Degree of Utilization

$$\eta v = FvEd / FvRd = 0,10 \leq 1$$

$$\eta t = FtEd / FtRd = 0,24 \leq 1$$

$$\eta = \eta v + \eta t / 1,4 = 0,27 \leq 1$$

$$\eta p = FtEd / BpRd = 0,06 \leq 1$$

specific pressure on hole

edge distance longitudinal to force...  $e1$  = 30 mm

bolt gap transversal to force...  $e2$  = 30 mm

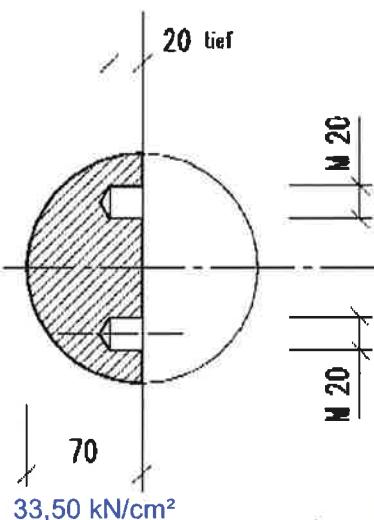
bolt gap longitudinal to force...  $p2$  =  $eb * \cos(0)$  = 80 mm

$\alpha b$  =  $\text{MIN}(e1/(3*dh); fub/fu; 1,0)$  = 0,455

$k1$  =  $\text{MIN}(2,8*e2/dh-1,7; 1,4*p2/db-1,7; 2,5)$  = 2,118

$FbRd$  =  $k1 * \alpha b * fu * db * tp / \gamma M2 / 1000$  = 235,91 kN

$\eta$  =  $FvEd / FbRd$  = 0,04  $\leq 1$

**Half Section**

$$f_{ys} = 335,0 / 10 = 33,50 \text{ kN/cm}^2$$

**shear + torsional moment**

$$r = \frac{d_0}{2} = 69,85 \text{ mm}$$

$$t_{\text{notional}} = 5,00 \text{ mm}$$

$$A_m = 0,5 * \pi * (r - 0,5 * t_{\text{notional}})^2 \approx 125 \text{ mm}^2$$

$$\Rightarrow W_T = 2 * A_m * t_{\text{notional}} / 1000 = 71,25 \text{ cm}^3$$

$$\tau_T = 100 * \frac{T_{Ed}}{W_T} = 4,41 \text{ kN/cm}^2$$

$$\tau_v = 1,5 * \text{MAX}(V_{yEd}; V_{zEd}) / A_m = 0,002 \text{ kN/cm}^2$$

$$\tau_{Ed} = \tau_T + \tau_v = 4,41 \text{ kN/cm}^2$$

$$\tau_{Rd} = f_{ys} / (\gamma M_0 * \sqrt{3}) = 19,34 \text{ kN/cm}^2$$

$$\eta = \tau_{Ed} / \tau_{Rd} = 0,23 \leq 1$$

**Tension and Bending**

$$r_i = r - t_{\text{notional}} = 64,85 \text{ mm}$$

$$A = 0,5 * \pi * \frac{r_i^2}{100} = 66,06 \text{ cm}^2$$

$$W_o = 0,191 * \frac{r_i^3}{1000} = 52,09 \text{ cm}^3$$

$$M_y = V_{yEd} * 8,1 = 42,61 \text{ kNm}$$

$$W_z = \frac{1}{8} * \frac{r_i^3}{1000} = 34,09 \text{ cm}^3$$

$$M_z = V_{zEd} * 8,1 = 88,05 \text{ kNm}$$

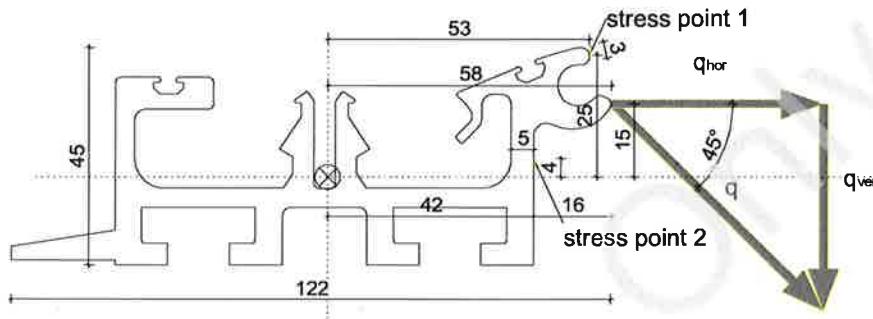
$$\sigma_d = \frac{NEd}{A} + \frac{My}{W_o} + \frac{Mz}{W_z} = 3,65 \text{ kN/cm}^2$$

$$\eta = \sigma_d / (f_y s / \gamma M_0) = 0,11 < 1$$

**Pos. 34 skipped****Pos. 35 skipped**

## Pos. 36 Extrusion - F16.2

## System



load eccentricity  $e_y = 5,80 \text{ cm}$   
load eccentricity  $e_z = 1,50 \text{ cm}$

## **Material**

Aluminium AW-6060 T66  
acc. to EN 1999-1-1:2010

0,2% offset yield strength $f_{o,alu}$ =	<b>15,00 kN/cm<sup>2</sup></b>
tensile strength $f_{u,alu}$ =	<b>19,50 kN/cm<sup>2</sup></b>

young' modulus  $E_{alu} = 70000,0 \text{ N/mm}^2$   
shear modulus  $G_{alu} = 27000,0 \text{ N/mm}^2$

material safety factor  $\gamma_{M1\text{ alu}} = 1,10$

$$\text{yield strength } f_{\text{alu,d}} = f_{\text{o,alu}} / \gamma_{M1, \text{alu}} = 13,64 \text{ kN/cm}^2$$

## Loads

## Determination of loads from the ultimate strength of the membrane layers

partial load safety factor...  $\gamma_F =$

$$\text{sum layer thicknesses } t_m = 0,20+0,20 = 0,40 \text{ mm}$$

(middle + bottom layer, see part 2)

allowable stress  $f_{all}$  = 12,0 N/mm<sup>2</sup>

max. slope of lower layer...  $\alpha$  = 45,0°

$$\text{tensile force per meter... } q_d = \gamma F * t_m * f_{all} = 7,20$$

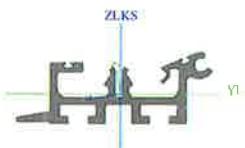
$$\text{horizontal force } q_{\text{hor},d} = q_d * \cos(\alpha) = 5.09 \text{ kN/m}$$

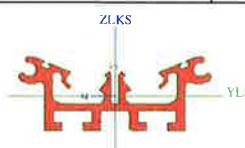
$$\text{vertical force } a_{11} = a_{11} * \sin(\alpha) = 5.00 \text{ kN/m}$$

**Note.**  
**Maxim**

Maximum stresses of 12,0 N/mm<sup>2</sup> are relevant for long term load. In general these loads are acting on two layers (i. e. snow load). The subsequently used load "q" covers the maximum tensile membrane force of 12,0 N/mm<sup>2</sup> x 0,4 mm = 4,80 N/mm for snow load and 18,0 N/mm<sup>2</sup> x 0,2 mm = 3,6 kN/m or wind load acting on the upper layer.

Cross section

Name	F16.2 Single	
Typ	Allgemeiner Querschnitt	
Materialangabe	S 235	
Herstellung	allgemein	
Knick y-y, z-z	c	c
FEM-Analyse	✓	
		
<b>A [cm<sup>2</sup>]</b>	14,61	
<b>A y, z [cm<sup>2</sup>]</b>	1,04	3,48
<b>I y, z [cm<sup>4</sup>]</b>	160,89	19,30
<b>I YLCS, ZLCS [cm<sup>4</sup>]</b>	21,64	158,55
<b>I w [cm<sup>6</sup>], t [cm<sup>4</sup>]</b>	280,71	1,23
<b>W<sub>pl</sub> y, z [cm<sup>3</sup>]</b>	24,26	7,47
<b>W<sub>pl</sub> y, z [cm<sup>3</sup>]</b>	42,09	13,96
<b>d y, z [mm]</b>	-9,89	3,17
<b>c YLKS, ZLKS [mm]</b>	3,62	-4,24
<b>Alpha [deg]</b>	97,39	
<b>IYZLKS [cm<sup>4</sup>]</b>	18,06	
<b>AL [m<sup>2</sup>/m]</b>	0,65	
<b>M<sub>ply +, -</sub> [Ncm]</b>	1,01e+06	1,01e+06
<b>M<sub>plz +, -</sub> [Ncm]</b>	3,35e+05	3,35e+05

Name	F16.2 Double	
Typ	Allgemeiner Querschnitt	
Materialangabe	S 235	
Herstellung	allgemein	
Knick y-y, z-z	c	c
FEM-Analyse	✓	
		
<b>A [cm<sup>2</sup>]</b>	14,87	
<b>A y, z [cm<sup>2</sup>]</b>	2,82	5,15
<b>I y, z [cm<sup>4</sup>]</b>	164,88	22,27
<b>I YLCS, ZLCS [cm<sup>4</sup>]</b>	22,27	164,88
<b>I w [cm<sup>6</sup>], t [cm<sup>4</sup>]</b>	241,98	1,24
<b>W<sub>pl</sub> y, z [cm<sup>3</sup>]</b>	28,25	9,12
<b>W<sub>pl</sub> y, z [cm<sup>3</sup>]</b>	43,44	15,90
<b>d y, z [mm]</b>	-15,75	0,00
<b>c YLKS, ZLKS [mm]</b>	0,00	-1,96
<b>Alpha [deg]</b>	90,00	
<b>IYZLKS [cm<sup>4</sup>]</b>	0,00	
<b>AL [m<sup>2</sup>/m]</b>	0,67	
<b>M<sub>ply +, -</sub> [Ncm]</b>	1,04e+06	1,04e+06
<b>M<sub>plz +, -</sub> [Ncm]</b>	3,82e+05	3,82e+05

⇒ values for extrusion type "F16.2 single" will be used

$$I_y = 21,64 \text{ cm}^4$$

$$I_z = 158,55 \text{ cm}^4$$

$$I_t = 1,23 \text{ cm}^4$$

design**double-span beam:**

$$M_{y,d} = q_{vert,d} * e_{up}^2 * 1/8 / 100 = 63,6 \text{ kNm}$$

$$M_{z,d} = q_{hor,d} * e_{up}^2 * 1/8 / 100 = 63,6 \text{ kNm}$$

$$M_{t,d} = (q_{hor,d} * e_z + q_{vert,d} * e_y) * e_{up}/2 / 100 = 18,6 \text{ kNm}$$

**stress point 1**

$$W_y = I_y / 2,50 = 8,7 \text{ cm}^3$$

$$W_z = I_z / 5,30 = 29,9 \text{ cm}^3$$

$$W_t = I_t / 0,30 = 4,1 \text{ cm}^3$$

$$\sigma_d = M_{y,d} / W_y + M_{z,d} / W_z = 9,4 \text{ kN/cm}^2$$

$$\tau_d = M_{t,d} / W_t = 4,5 \text{ kN/cm}^2$$

$$\text{v. Mises } \sigma_v = \sqrt{(\sigma_d^2 + 3 * \tau_d^2)} = 12,2 \text{ kN/cm}^2$$

$$\text{coeff. of utilization } \eta = \sigma_v / f_{alu,d} = 0,89 \leq 1$$

**stress point 2**

$$W_y = I_y / 0,40 = 54,1 \text{ cm}^3$$

$$W_z = I_z / 4,20 = 37,8 \text{ cm}^3$$

$$W_t = I_t / 0,50 = 2,5 \text{ cm}^3$$

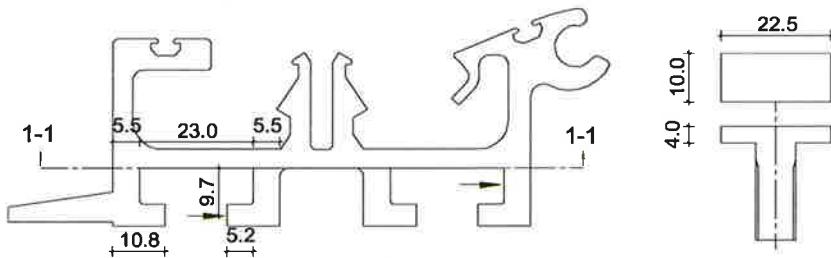
$$\sigma_{x,d} = M_{y,d} / W_y + M_{z,d} / W_z = 2,86 \text{ kN/cm}^2$$

$$\sigma_{z,d} = q_{vert,d} / (0,5 * 100) + (q_{hor,d} * 1,50 + q_{vert,d} * 1,6) / 100 / (0,50^2 / 6) = 3,89 \text{ kN/cm}^2$$

$$\tau_d = 0,5 * q_{vert,d} * e_{up} / (3 * 0,5 * 4,0 * 100) + M_{t,d} / W_t = 7,86 \text{ kN/cm}^2$$

$$\text{v. Mises } \sigma_v = \sqrt{(\sigma_{x,d}^2 + \sigma_{z,d}^2 - \sigma_{x,d} * \sigma_{z,d} + 3 * \tau_d^2)} = 14,05 \text{ kN/cm}^2$$

$$\text{coeff. of utilization } \eta = \sigma_v / f_{alu,d} = 1,03 \leq 1$$

Local stress check at extrusion**section 1-1: Contact point at bolt shank**

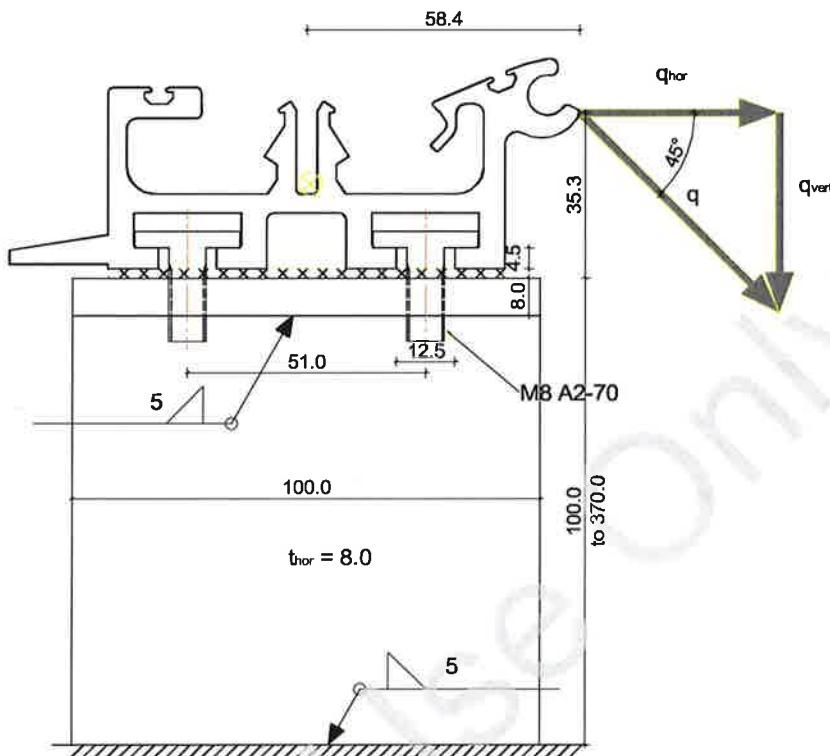
number of bolts $n_b$ =	4
$h_{Ex}$ =	0,970 cm
$t_{Ex}$ =	0,550 cm
$b_{Ex}$ = $1,08 - t_{Ex}/2$	= 0,805 cm
<i>effective width, with angle of load-spreading = 45°</i>	
$b_{eff,Ex}$ = $2 * (b_{Ex} + h_{Ex})$	= 3,550 cm
moment of resistance $W$ = $1,25 * b_{eff,Ex} * t_{Ex}^2 / 6$	= 0,2237 cm <sup>3</sup>
acting force, per bolt $F_d$ = $1,25 * q_{hor,d} * e_{up}/100 / n_b$	= 1,591 kN
stress $\sigma_d$ = $F_d * h_{Ex} / W$	= 6,90 kN/cm <sup>2</sup>
coeff. of utilization $\eta$ = $\sigma_d / f_{alu,d}$	= <u>0,51 ≤ 1</u>

**section 1-1: Contact point between bolt head and vertical web**

bolt head hight $t_b$ =	0,400 cm
bolt head width $b_b$ =	1,000 cm
<i>effective width, with angle of load-spreading = 45°</i>	
$b_{eff,Ex}$ = $2 * h_{Ex} + b_b$	= 2,940 cm
moment of resistance $W$ = $1,25 * b_{eff,Ex} * t_{Ex}^2 / 6$	= 0,1853 cm <sup>3</sup>
acting force, per bolt $F_d$ = $1,25 * q_{hor,d} * e_{up}/100 / n_b$	= 1,591 kN
stress $\sigma_d$ = $F_d * h_{Ex} / W$	= 8,33 kN/cm <sup>2</sup>
coeff. of utilization $\eta$ = $\sigma_d / f_{alu,d}$	= <u>0,61 ≤ 1</u>

## Pos. 37 Upstands

### Material and mechanical System



#### **Hammer-head bolts M8x30 A2-70**

number of bolts $n_b =$	4
max. hor. capacity $V_{Rd} =$	8,70 kN
hole diameter $d_0 =$	1,25 cm
bolt diameter $d =$	0,80 cm
eff. thread cross section area $A_{Sp} =$	0,370 cm <sup>2</sup>
partial safety factor $\gamma_{M2} =$	1,25
tensile strength bolt $f_{ub} =$	70,00 kN/cm <sup>2</sup>

steel grade S235 according to EN 10025-2

$$f_{y,d} = 23,5 / \gamma_{M0} = 23,5 \text{ kN/cm}^2$$

$$f_u = \frac{360}{10} = 36,0 \text{ kN/cm}^2$$

#### **Geometry**

$$\text{max. clearance between upstands } e_{up} = e_{up} = 100,00 \text{ cm}$$

$$\text{clearance between bolts } e_b = 5,10 \text{ cm}$$

$$\text{load eccentricity } e_1 = 5,84 \text{ cm}$$

$$\text{load eccentricity } e_2 = 3,53 \text{ cm}$$

$$\text{max. height of upstands } h_{up} = 10,00 \text{ cm}$$

$$\text{thickness upstand plate } t_{up} = 0,80 \text{ cm}$$

$$\text{thickness horizontal plate } t_{hor} = 0,80 \text{ cm}$$

$$\text{width upstand plate } b_{up} = 10,00 \text{ cm}$$

**Loads**

Determination of loads from the ultimate strength of the membrane layers

sum layer thickness  $t_m = 0,20+0,20 = 0,40 \text{ mm}$

allowable stress  $f_{all} = 12,0 \text{ N/mm}^2$

max. slope of lower layer...  $\alpha = 45,0^\circ$

tensile force per meter...  $q_d = \gamma F * t_m * f_{all} = 7,20 \text{ kN/m}$

horizontal force  $q_{hor,d} = q_d * \cos(\alpha) = 5,09 \text{ kN/m}$

vertical force  $q_{vert,d} = q_d * \sin(\alpha) = 5,09 \text{ kN/m}$

load component parallel to the extrusion:

horizontal force  $q_{hor,II,d} = e_{up}/100 * 0,20 = 0,20 \text{ kN/m}$

**Note:**

Maximum stresses of 12,0 N/mm<sup>2</sup> are relevant for long term loads. In general these loads are acting on two layers (i. e. load). The subsequently used load "q" covers the maximum tensile membrane force of 18,0 N/mm<sup>2</sup> x 0,20 mm = 3,6 N/mm due to wind suction.

**Design of upright plate**

Area  $A = b_{up} * t_{up} = 8,00 \text{ cm}^2$

moment of resistance  $W_{y,el} = b_{up}^2 * t_{up} / 6 = 13,33 \text{ cm}^3$

moment of resistance  $W_{z,el} = b_{up} * t_{up}^2 / 6 = 1,07 \text{ cm}^3$

$M_{y,d} = (q_{hor,d} * (h_{up} + e_2) + q_{vert,d} * e_1) * e_{up} / 100 = 98,59 \text{ kNm}$

$M_{z,d} = (q_{hor,II,d} * (h_{up} + e_2) + q_{vert,d} * t_{up}/2) * e_{up} / 100 = 4,74 \text{ kNm}$

$\sigma_d = q_{vert,d}/100 * e_{up}/A + M_{y,d}/W_{y,el} + M_{z,d}/W_{z,el} = 12,46 \text{ kN/cm}^2$

$\tau_d = \sqrt{(q_{hor,d}^2 + q_{hor,II,d}^2) * e_{up} / (100 * A)} = 0,64 \text{ kN/cm}^2$

v. Mises  $\sigma_v = \sqrt{(\sigma_d^2 + 3 * \tau_d^2)} = 12,51 \text{ kN/cm}^2$

coeff. of utilization  $\eta = \sigma_v / f_{y,d} = 0,53 \leq 1$

plate has to be welded to substructure by fillet-welds

throat dimension  $a = 4 \text{ mm}$  (nominal leg D = 5,6 mm)design:  $2 \times 4,0 = 8 \text{ mm} = 1,0 \times t_{up}$ 

s235...  $\beta_w = 0,80$

$f_{weld} = f_u / \sqrt{3} / (\beta_w * \gamma M2) = 20,78 \text{ kN/cm}^2$

coeff. of utilization  $\eta = \sigma_v / f_{weld} = 0,60 \leq 1$

**Bolted connection between extrusion and horizontal plate**

$$\text{horizontal force per bolt } F_{v,d} = \sqrt{(q_{\text{hor},d}^2 + q_{\text{hor,II},d}^2) / (100 * n_b)} * e_{\text{up}} = 1,27 \text{ kN}$$

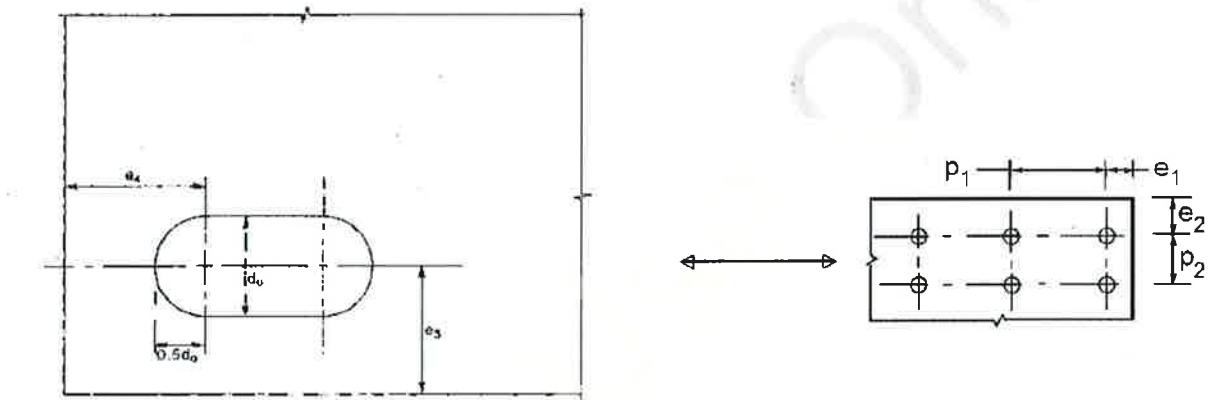
$$\text{vertikal force per bolt } F_{t,d} = (q_{\text{hor},d} * e_2 + q_{\text{vert},d} * e_1) / (e_b * n_b / 2) = 4,68 \text{ kN}$$

**resistance according to EN 1993-1-4:2006****single shear**

$$\text{shear joint in thread } \alpha = \text{MIN}(V_{\text{Rd}}; \alpha * A_{\text{Sp}} * f_{\text{ub}} / \gamma_{M2}) = 0,50 \text{ kN}$$

**tensional force**

$$k_2 = \text{MIN}(k_2 * A_{\text{Sp}} * f_{\text{ub}} / \gamma_{M2}) = 0,90 \text{ kN}$$

**specific pressure on hole**

$$\text{min. edge distance in load direction } e_1 = 1,2 * d_0 = 1,50 \text{ cm}$$

$$\text{min. edge distance orthogonal to load direction } e_2 = 1,2 * d_0 = 1,50 \text{ cm}$$

$$\text{min. edge distance in load direction (elongated hole) } e_3 = 1,5 * d_0 = 1,88 \text{ cm}$$

$$\text{min. edge distance orthogonal to load direction (elongated hole) } e_4 = 1,5 * d_0 = 1,88 \text{ cm}$$

$$\text{min. bolt distance in load direction } p_1 = 2,2 * d_0 = 2,75 \text{ cm}$$

$$\text{min. bolt distance orthogonal to load direction } p_2 = 2,4 * d_0 = 3,00 \text{ cm}$$

$$k_1 = \text{MIN}(2,5 ; 2,8 * e_2 / d_0 - 1,7; 1,4 * p_2 / d_0 - 1,7) = 1,66$$

**horizontal plate:**

$$t_{\text{hor}} = 0,80 \text{ cm}$$

$$\alpha_b = \text{MIN}(e_1 / (3 * d_0); p_1 / (3 * d_0) - 1/4; f_{\text{ub}} / f_u; 1,0) = 0,400$$

(for elongated hole = 0,60)

$$F_{b,Rd,hor} = 0,60 * k_1 * \alpha_b * f_u * d * t_{\text{hor}} / \gamma_{M2} = 7,34 \text{ kN}$$

**extrusion profile:**

(acc. to DIN EN 1999-1-1 = 0,80 for aluminium)

$$t_{\text{Ex}} = 0,45 \text{ cm}$$

$$\alpha_b = \text{MIN}(e_1 / (3 * d_0); p_1 / (3 * d_0) - 1/4; f_{\text{ub}} / f_u; 0,66) = 0,400$$

$$F_{b,Rd,Ex} = 0,80 * k_1 * \alpha_b * f_u * d * t_{\text{Ex}} / \gamma_{M2} = 5,51 \text{ kN}$$

**check due to shear forces**

$$F_{v,d} = 1,27 \text{ kN}$$

$$F_{v,Rd} = 8,70 \text{ kN}$$

$$\eta_v = F_{v,d} / F_{v,Rd} = 0,15 \leq 1$$

**check due to tensional forces**

$$\begin{aligned}
 F_{t,d} &= 4,68 \text{ kN} \\
 F_{t,Rd} &= 18,60 \text{ kN} \\
 \eta_t = F_{t,d} / F_{t,Rd} &= \underline{0,25 \leq 1}
 \end{aligned}$$

**check due to specific pressure on hole***horizontal plate:*

$$\eta_{b,hor} = F_{v,d} / F_{b,Rd,hor} = \underline{0,17 \leq 1}$$

*extrusion profile:*

$$\eta_{b,Ex} = F_{v,d} / F_{b,Rd,Ex} = \underline{0,23 \leq 1}$$

**interaction**

$$\eta_{v+t} = F_{v,d} / F_{v,Rd} + F_{t,d} / (1,4 * F_{t,Rd}) = \underline{0,33 \leq 1}$$

**Annex - Structural Analysis Printout**

Part	Annex	Rev	Subject	Pages
	A	new	ETFE - downward loads	21
	B	new	Structural Steel - lifting loads	21
	C	new	Structural Steel - downward loads	25

D-Bremen, August 14, 2013  
 Dr.-Ing. Tobias Schween

To: **Jackson Contractor Group** Date: **4/5/2013**  
**Mike Chase**

CC: **WB+P**

Project Name: **Parmly Billings Library** Project Number: **1101**

We transmit:

Herewith  
 In accordance with your request  
 Under separate cover via:

For Your

Approval       Information       Distribution to Others  
 Review & Comment       Record       \_\_\_\_\_

The following:

Drawings       Shop Drawings       See Below  
 Specifications       Product Literature       Digital Files

Copies      Date:      Description:

1      08 6300 -1 (Phase 1) ETFE      **Approved**

Remarks

**First of 3 submittals. Need to discuss possibility of offsetting tube to interior to avoid existing wall**



## Submittal Transmittal

Detailed, Grouped by Each Number

Parmly Billings Library  
510 N. Broadway  
Billings, MT 59101

Project # 2012.35  
Tel: 406-542-9150 Fax: 406-542-3515

Jackson Contractor Group Inc.

Date: 4/2/2013

Reference Number: 0174

Transmitted To:	Don Olsen O2 Architects 208 N. Broadway Suite 350 Billings, MT 59101 Tel: 406-259-7123 Fax: 406-256-7123	Transmitted By:	Mike Chase Jackson Contractor Group Inc. P.O. Box 967 Missoula, MT 59806 Tel: 406.542.9150 Fax: 406.542.3515
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Qty	Submittal Package No	Description	Due Date	Package Action
1	0054 - 08 6300 - 2	Metal Framed Skylight - ETFE Skylight	4/8/2013	Submitted

Transmitted For	Delivered Via	Tracking Number
Approval	e-mail	

Items	Qty	Description	Notes	Item Action
0001	1	Mtl Frame Skylight - Calculations		Submitted
0002	0	Mtl Frame Skylight - Drawings		Pending
0003	0	Mtl Frame Skylight - Samples		Pending

Cc: Company Name	Contact Name	Copies	Notes
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**Remarks**

Don,  
Please see the below for sequence of necessary submittal review and approval. Submittals will be transmitted in this sequence to complete the total specification requirements.  
April 2nd: Submit load calculations and connection requirements based on the redesign of the structural steel for the skylight system. Our team will be available to address any concerns and explain the modifications.  
April 8th: Submit steel shop drawings and provide necessary assistance for approval.  
April 16th: Submit Texlon System drawings

Thanks,  
Mike

MIKE CHASE

Signature

4.2.13

Signed Date



**Jackson**  
**CONTRACTOR GROUP, INC.**  
P.O. Box 967, Missoula, MT 59806  
406-542-9150 | FAX 542-3515

Summary with Register Items & Stamp

Project # 2012.35

**Parmly Billings Library**  
510 N. Broadway  
Billings, MT 59101

**Jackson Contractor Group Inc.**  
Tel: 406-542-9150 Fax: 406-542-3515

Item No	Register No	Rev	Spec Section	Sub Section	Description	Responsible	Supplier	Rec'd On	Action
<b>0054 - 08 6300 - 2</b>									
0001	00194	2	08 63 00	1.03.A	Mt Frame Skylight - Calculations	Vectorfolite, LLC	Texlon	4/2/2013	Submitted
0002	00195	2	08 63 00	1.03.B	Mt Frame Skylight - Drawings	Vectorfolite, LLC	Texlon	4/2/2013	Pending
0003	00196	2	08 63 00	1.03.C	Mt Frame Skylight - Samples	Vectorfolite, LLC	Texlon	4/2/2013	Pending

**SUBMITTAL REVIEW**

REVIEWED, NO EXCEPTIONS TAKEN     REVISE AND RESUBMIT  
 NOTE COMMENTS     SEE ATTACHED COMMENTS

Corrections or comments made to the shop drawings during this review do not relieve subcontractor/vendor from compliance with requirements of the drawings and specifications. This check is only for review or general conformance with the design concept of the project and general compliance with the information given in the contract documents. The subcontractor/vendor is responsible for confirming and correlating all quantities and dimensions, selecting fabrication processes and techniques of construction, coordinating his work with that of all other trades, and performing his work in a safe and satisfactory manner.

**JACKSON CONTRACTOR GROUP, INC.**

BY: **MIKE CHASE**

DATE: **4.2.13**

**rudow + berry**  
**structural engineers**

This review was performed only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Modifications or comments made on the shop drawings during this review do not relieve the contractor from compliance with the requirements of the plans and specifications. Approval of a specific item does not include approval of the assembly of which the item is a component.

Contractor is Responsible for:

- \* Dimensions to be confirmed and correlated at the jobsite.
- \* Information that pertains solely to the fabrication processes or to the means, methods, techniques, sequences and procedures of construction.
- \* Coordination of the work of all trades.
- \* Performing all work in a safe and satisfactory manner.

Approved     Approved As Corrected  
 Reviewed For Loading Only

Revise and Resubmit     Rejected  
 Resubmit Record Copy

Reviewed By: **MAR** Date: **04/05/2013**

Architect Please Note: Please verify that revised ring beam size (TS14x6) is acceptable.

**yes**

Vector Foiltec • 13 Green Mountain Drive • Cohoes • New York 12047 USA

**March 26, 2013**

**Jackson Contractor Group, INC.**  
**ATTN: Mike Chase**

13 Green Mountain Drive  
Cohoes, New York  
12047 USA  
T +1 518 783 0575  
F +1 518 783 0474  
us@vector-foiltec.com  
www.vector-foiltec.com

**RE: Parmly Billings Library - Notice of Submittal Delay**

Mike,

**A/E - FYI see proposed structural steel solution enclosed**

Vector Foiltec's original schedule required the shop drawings to be approved by 11/30/2012 to meet the completion date of 4/4/2013 (17 weeks after approval). As discussed previously, we will not be able to meet the hand-over date by the beginning of April. At this time all shop drawings and samples have been submitted, with the exception of the steel and Texlon shop drawings, which are dependant on the approved structural calculations.

The extended duration for resubmitting the necessary information was the result of the following activities:

The original design called for a 5" tubular ring beam to be welded to 16 upright posts. The architect's intent was for the skylight structure to transmit vertical loads to the posts without imposing additional horizontal loads. Based on the connection type, and the tubing size specified, the skylight structure was found to transmit unintended loads to the posts. Our engineer from TSS in Germany, Dr. Schween, first noted that the posts could not take the calculated loads and advised that their wall thickness be increased. Due to the connection detail described in the structural drawings, he also determined that there was still a limit to the post's capacity. Based on these limitations, he calculated for the climactic loads using the ASCE tables, in order to prevent the posts from being overstressed. It was also suggested that the structural steel may require re-design.

Vector Foiltec has concluded that re-design is necessary for the skylight structural steel in order to balance the loads among the support posts. The 16 support posts for the skylight are already supplied and installed. Grid Engineering, Seattle Washington, is currently specifying the required ring beam material and a connection detail between it and the posts. They will be providing the stamped calculations and drawings for review. These calculations with the modified structure are necessary to complete the steel and Texlon shop drawings. We have made every effort to resolve this issue without affecting the design, however the originally specified components and the provided posts limit this capability. This can only be accomplished by increasing the size of the steel ring beam and modifying the connection between it and the posts.



April 01, 2013

Mr. Philipp Lehnert  
Project Manager  
Vector Foiltec  
13 Green Mountain Drive  
Cohoes, NY 12047

Re: Parmly Library  
Main ETFE Roof Skylight

Dear Philipp,

Thank you for the opportunity to work with you on the Parmly Library project.

Attached please find the calculation package for the steel design and loads imposed on the existing roof structure. This package is in coordination with the ETFE designers in Bremen. Geometry and various design details for the existing structure were found within the set you forwarded us (01May 2012, 100CD set, 14 structural sheets s1.0-s7.0). If you determine any condition in the field varies from that what is stated here, please contact us before proceeding.

Please have the EOR contact us directly if there are any questions with the information during their review. We appreciate the opportunity to work with you on this project. If you have any questions, please feel free to call us at 206.838.4000.

Sincerely,

Bryan Tokarczyk  
Partner

blt:psd:file

Enclosures  
120059.10

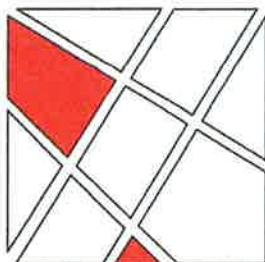


grid  
box 30797  
seattle 98113

Vector Foiltec  
Parmly Library  
[ Main ETFE Skylight Assembly ]  
Billings, MT

## Structural Calculations

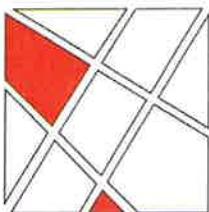
Calculations Included:  
These calculations cover the proposed skylight assembly loads imposed  
to the main facility roof



Project Number 120059.00  
April 01, 2013

# Enclosure One





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engineers  
seattle, wa  
206.838.4000

Project	PARMLY LIBRARY	
	- ETFE SKYLIGHT	
Job #	120059	
Eng 1		done
Eng 2		
Arch		

## ► Main skylight / roof

### - schematic



### ENVIRONMENTAL LOADS

[described here]

### ETFE DESIGN

[ATTACHED AS REFERENCE]



### ETFE LOAD XPER

[described here]



### STEEL DESIGN

[described here]



### LOADS IMPOSED TO ROOF SUPPORTS

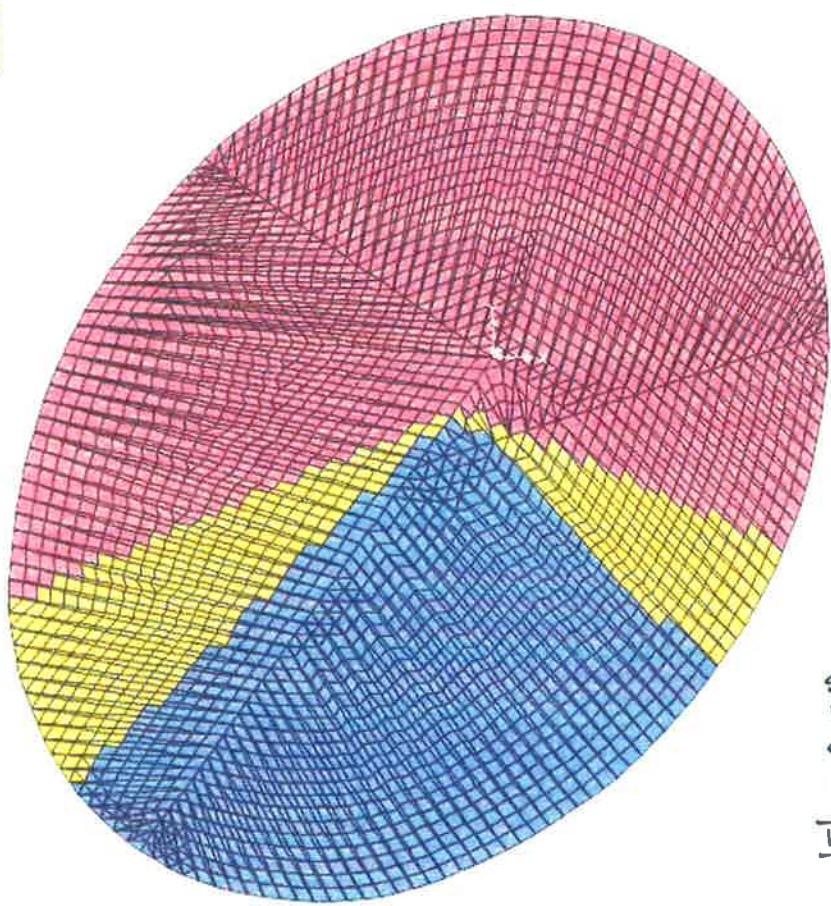
[described here]

- skylight system consists of steel frame with ETFE membrane cushions as environmental surface
- APPROX dimensions 24' x 32' ( $\approx 600 \text{ ft}^2$  AREA) ( $\approx 630 \text{ ft}^2$  SURFACE)
- ETFE  $\approx 100 \text{ #/ft}^2$   $\approx 10 \text{ mil thickness (.01")}$
- STEEL  $\approx 400 \text{ #/ft}^2$   $\approx 50 \text{ ksi (5)}$
- TAKE-OFF
 

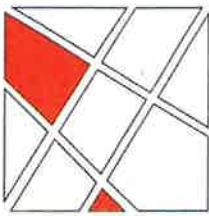
STEEL PERI	$2(25.5' + 18.75') = 87.5'$	lb	1660 #	2500	4640
STEEL CROSS	$(14.75' + 15.25' + 20') = 50.0'$	lb	950 #	1425	2650
ETFE FOIL	$(630 \text{ ft}^2 * .08 \text{ #/ft}^2) = 50 \text{ #}$	lb	137.5'	2600 #	3900
		lb			7300



BASIC STEEL FRAME  
(w/orientation)  
IMAGE A



SHELLED FRAME  
W/EXAMPLE DRIFT  
SNOW LOADING  
APPLIED  
IMAGE B



**GRID**

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project	job #
eng 1	date
eng 2	page

## ▷ MAIN SKYLIGHT / ROOF

### - LOAD COMBOS (IBC 09 / AT-5)

$$C1 = 1.4D + 1.4P_n$$

$$C3 = 1.2D + 1.2P_n + 1.6S + .8W$$

$$C4 = 1.2D + 1.2P_n + .5S + 1.6W$$

$$C6 = .9D + 1.4P_n + 1.6W$$

D = SELF STEEL + ETFE SYSTEM

P<sub>n</sub> = PNEUMATIC LOADING FROM ETFE SYSTEM UNDER LOAD

S = SNOW - BAL + UNBALANCE CASES

W = WIND - DOMED SURFACE

\* FROSTING & ICE LOADS CONSIDERED AND NOT CONTROL

(ALLOW DESIGN - ETFE STRESSING CONSIDER / PEPERS)

### - LOAD COMBOS (n = NORTH ATTACK WIND)

$$C1 = 1.4D + 1.4P_n$$

$$C3 = 1.2D + 1.2P_n + 1.6S + .8W$$

C3Sw<sub>n</sub>

C3Sw<sub>s</sub>

C3Sw<sub>e</sub>

C3Sw<sub>w</sub>

[UNIFORM]

C3Dw<sub>n</sub>

C3Dw<sub>s</sub>

C3Dw<sub>e</sub>

C3Dw<sub>w</sub>

[DRIFTING]

$$C4 = 1.2D + 1.2P_n + .5S + 1.6W$$

C4Sw<sub>n</sub>

C4Sw<sub>s</sub>

C4Sw<sub>e</sub>

C4Sw<sub>w</sub>

[UNIFORM]

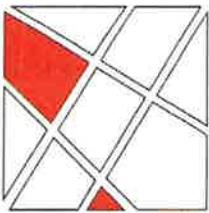
C4Dw<sub>n</sub>

C4Dw<sub>s</sub>

C4Dw<sub>e</sub>

C4Dw<sub>w</sub>

[DRIFTING]



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Project	Job #
Eng 1	date
Eng 2	page

## ▷ MAIN SKYLIGHT / ROOF

### - LOAD COMBOS (cont)

$$cb \quad .9D + 1.4P_n + 1.6W$$

cb  $W_n$   
cb  $W_s$   
cb  $W_e$   
cb  $W_w$

### - WIND

- $V = 90 \text{ MPH (3 sec) (FIG 6-1)}$
- EXP. C /  $I_w = 1.15$  (CAT III)
- BUILDING ENVELOPE
- IMPACT RESISTANT GLAZING
- RIGID STRUCTURE
- SIMPLIFIED (METH 1)

$$\begin{aligned} P_{net} &= \lambda + P_{net,0} & (H=40') \text{ ZONE 1 / ROOF } \pm 0^\circ / \\ & & ECp = \pm .9 \\ & & = 1.49 (1.15) \{ 4.7, -13.3 \} \\ & & = \{ \cancel{8.1}, \cancel{-22.8} \} \end{aligned}$$

MIN PRESSURE OVERRIDE

△ USE  $P_{net} = \{ +10, -22.8 \} \text{ PSF}$

### - SNOW

•  $P_g = 20 \text{ PSF (FIG 7-1)}$

•

$$\begin{aligned} 4'/12' &= 18^\circ & \text{FLAT } (\alpha_f) \leq 5^\circ \\ 4'/16' &= 14^\circ & \triangle \text{ NOT FLAT, SLOPED...} \end{aligned}$$

- YET, CONSERVATIVE ESTIMATION IS FLAT DUE TO POSSIBLE FALLOWS - SNOW WILL ACCUMULATE, WARM ROOF EFFECT NOT APPLY (CONCERN.)

△ FLAT -  $P_f = .7 C_c C_t I P_g$

$$= .7 (1.0) (1.0) (1.1) (20 \text{ PSF}) = \underline{15.4 \text{ PSF}}$$

NOTE: MT RULE OVERRIDE - TAKE  $P_f = 30 \text{ PSF}$  ("ROOF LOADING"  $\geq 30 \text{ PSF}$ )



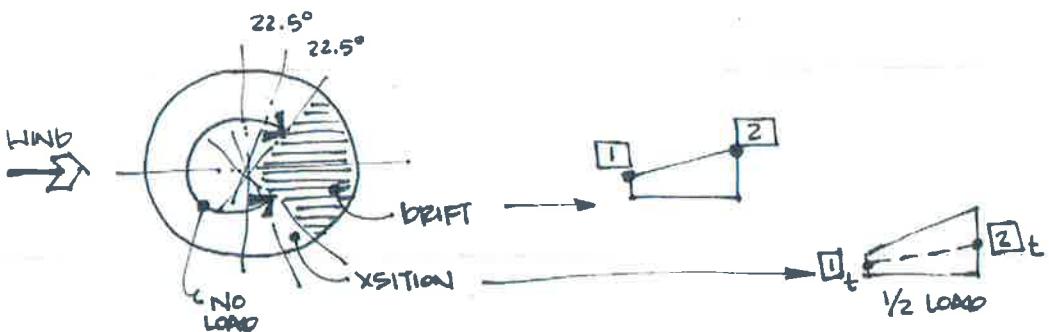
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seattle, wa  
206.838.4000

PROJECT	JOB #
ENGR 1	date
ENGR 2	page

## MAIN SKYLIGHT / ROOF

### Snow (cont.)

#### • UNBALANCED CONSIDERATION (7.6.4)(7.6.2)



$$C_g = .8 - 1.0 \text{ (FIG 7-2a)}$$

$$C_e = 1.0 \text{ (TABLE 7-2)}$$

**⚠ DESIGN UNBALANCED**

$$\boxed{1} .5 Pf = \frac{15.4}{2} = 7.7 \text{ PSF} \quad \text{30 PSF}$$

$$\boxed{2} 2.0 Pf \frac{C_g^*}{C_e}$$

$$2.0(15.4) \frac{.8}{1.0} = 24.6 \text{ PSF} \quad \text{30 PSF}$$

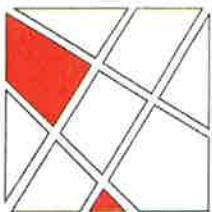
POSITION UNBALANCED

$$\boxed{1} t = \frac{7.7}{2} = 3.85 \text{ PSF} \quad \text{15 PSF}$$

$$\boxed{2} t = \frac{24.6}{2} = 12.3 \text{ PSF} \quad \text{15 PSF}$$

(OVERRIDE FOR MT RULE 30 PSF)  
MIN ROOF LOADING

• NOTE: LOADS IMPOSED AT TOP OF EXISTING TUBE POSTS (SEE DWS)



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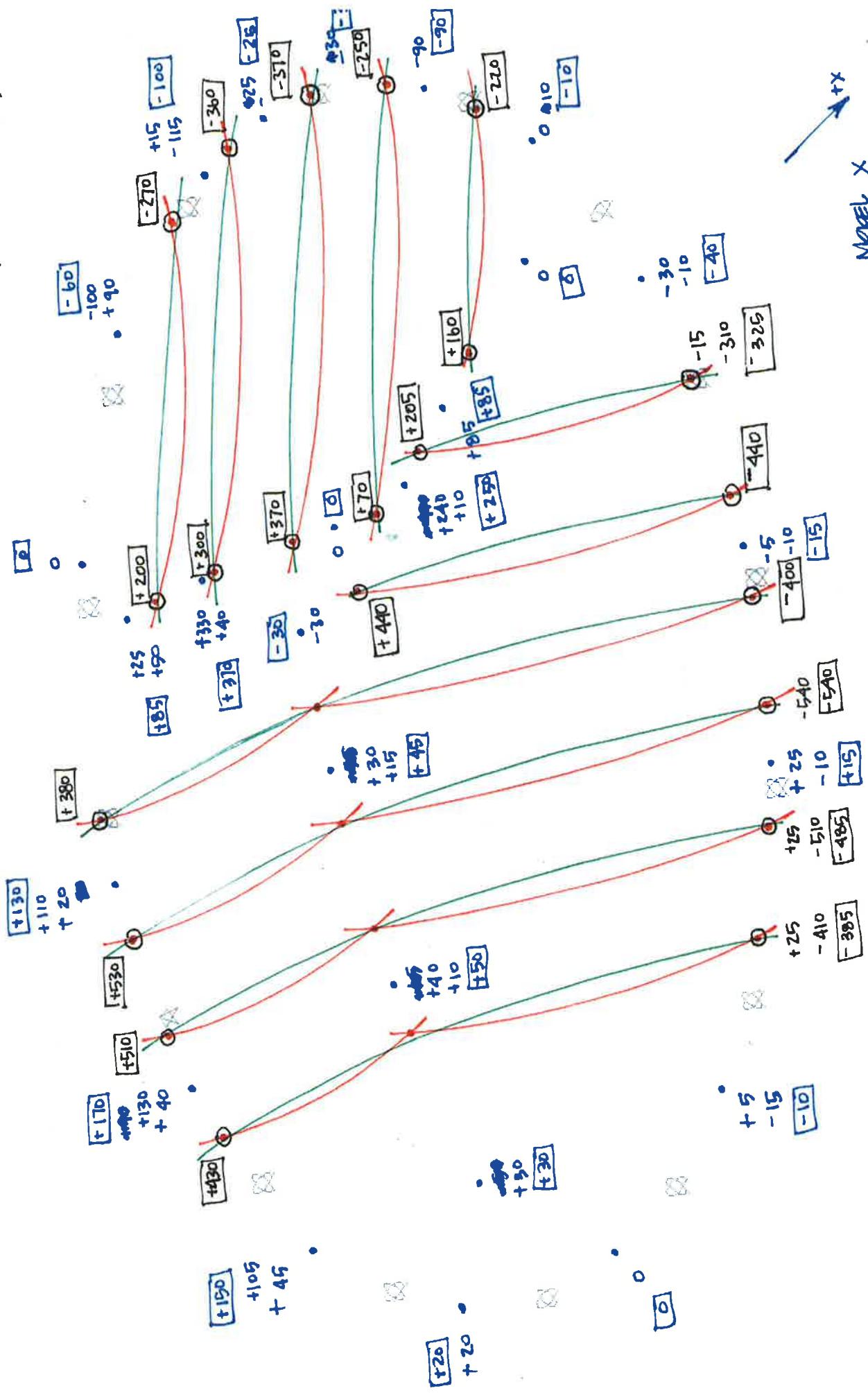
project	job #
enq 1	date
enq 2	page

## ▷ MAIN SKYLIGHT / ROOF

- FIXITY SCHEME - TARGET ± Z FIXITY ON ALL 16 SUPPORTS
  - RESTRAINT FOR STABILITY IN X + Y PER DIAGRAM
  - LOADS IMPOSED WILL REFLECT (NOTE: CHOICE OF RESTRAINED X,Y POSTS IS ARBITRARY AND CAN BE MOVED IF NEEDED, PLEASE CONTACT)
  - SEE IMAGE A
- ETFE PNEUMATIC XFR - SEE IMAGE C, D, E, F
- STEEL FRAME SIZING - SEE IMAGE G, H
- LOADS IMPOSED ON IMAGE J
- GLOBAL LOADS IMPOSED ON IMAGE K

TIGER EYE

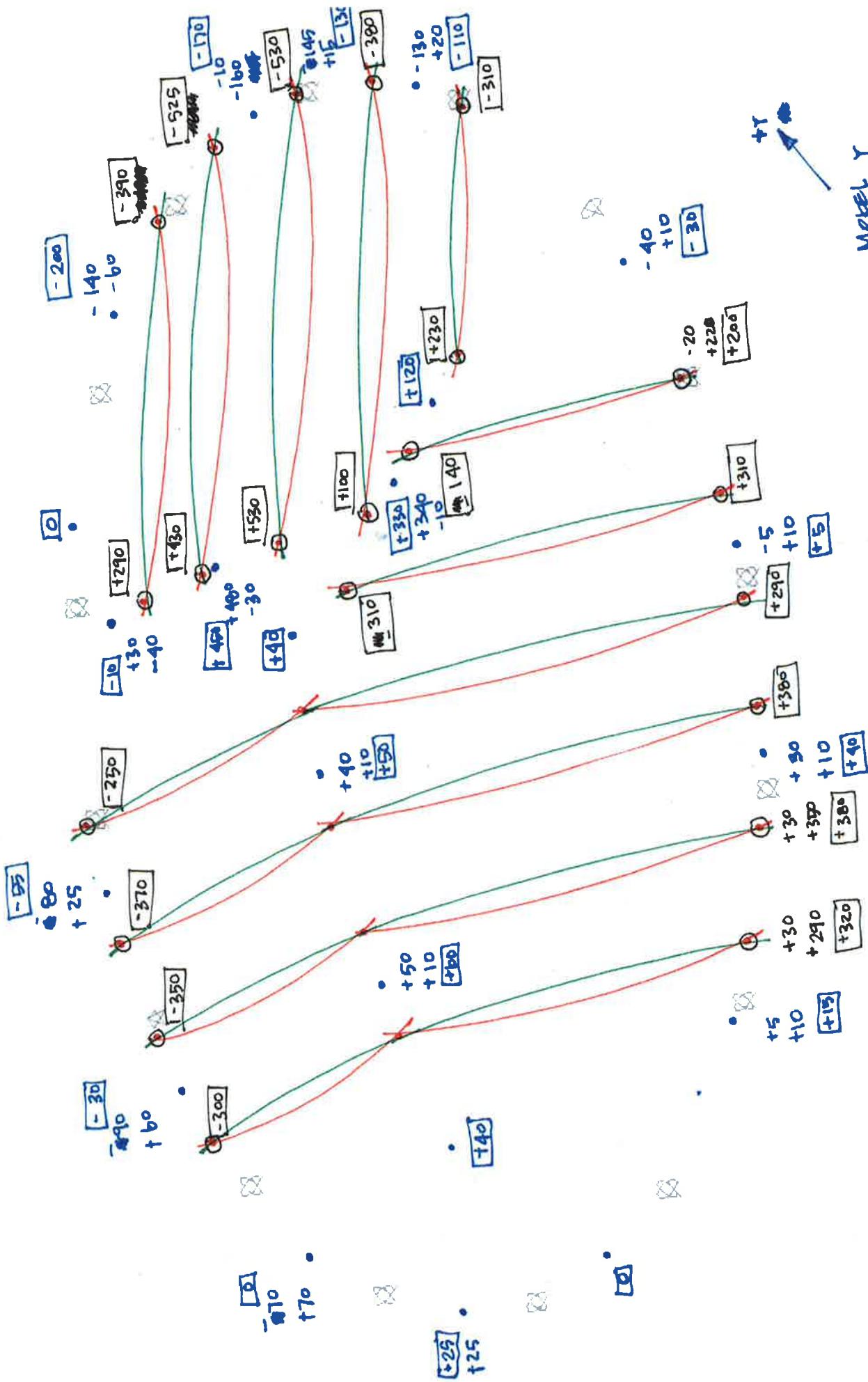
TIPS & TACTICS.NET



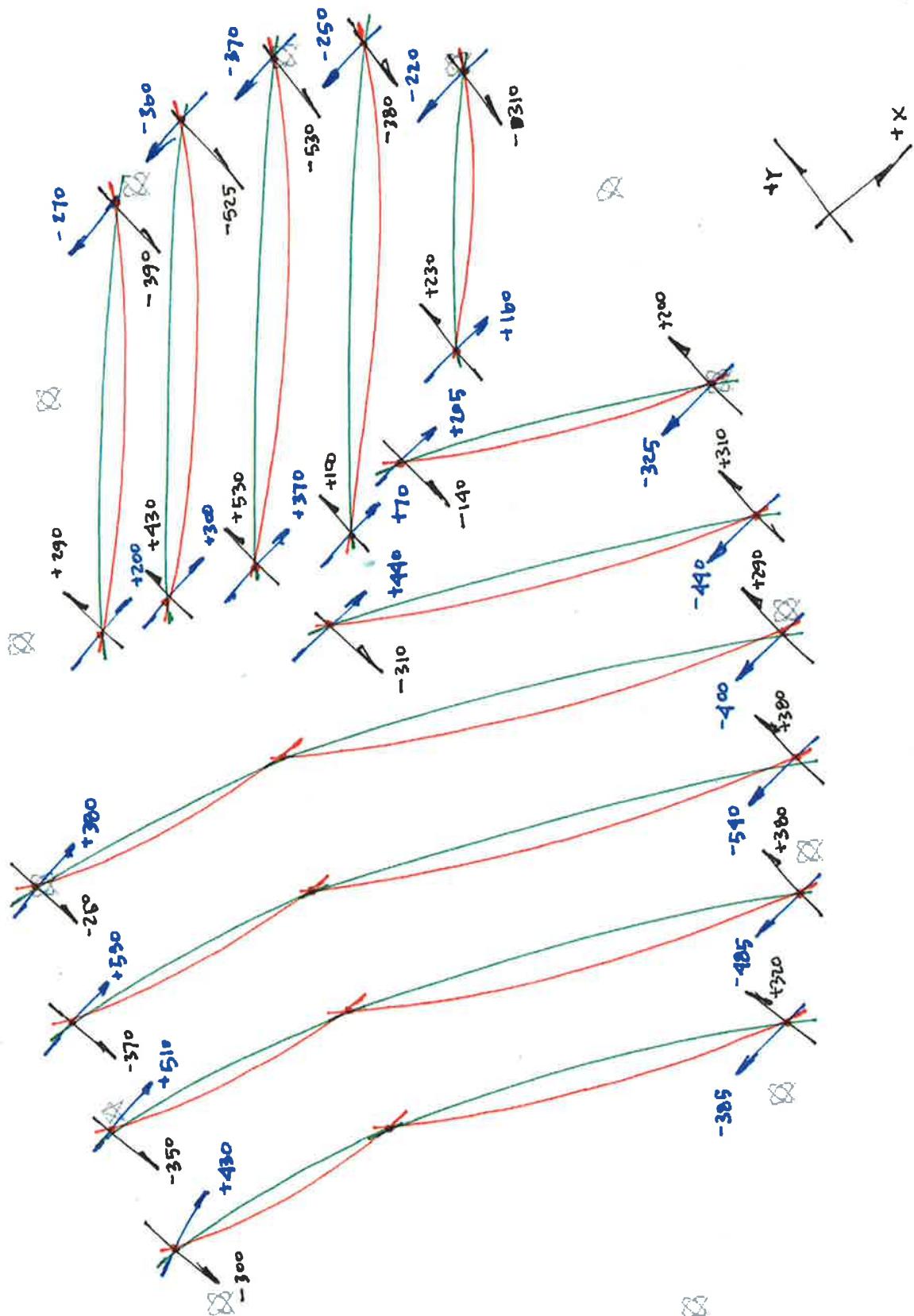
Model X

IMAGE C

THE. STRENGHTENING  
BIST. LOADS 163 (#14)  
POINT LOADS 163 (#14)



TEO. ETTERCAKE NET  
1900. 2112



IMAGE

IN. 212  
TAC. 345  
LENS 1 (X)  
LENS 2 (Y)  
LENS 3 (Z)  
#1 FT.

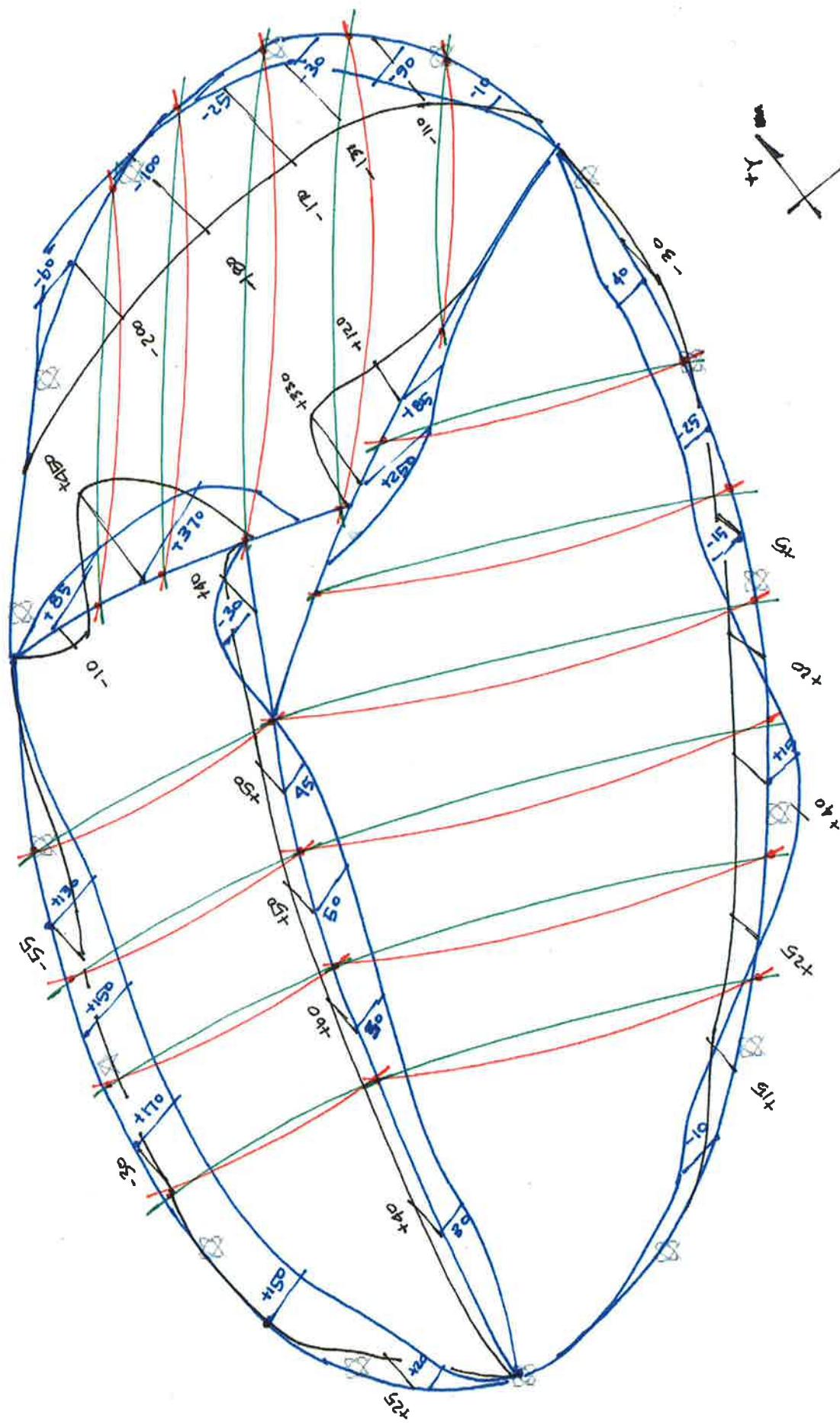


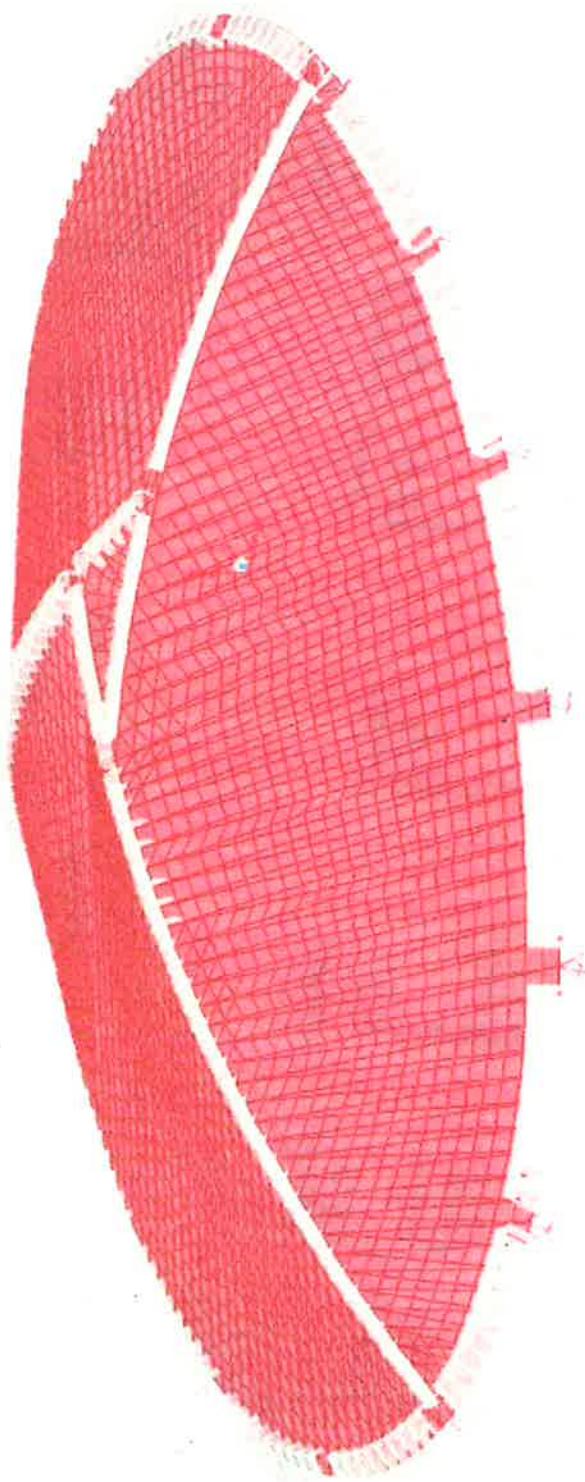
IMAGE F

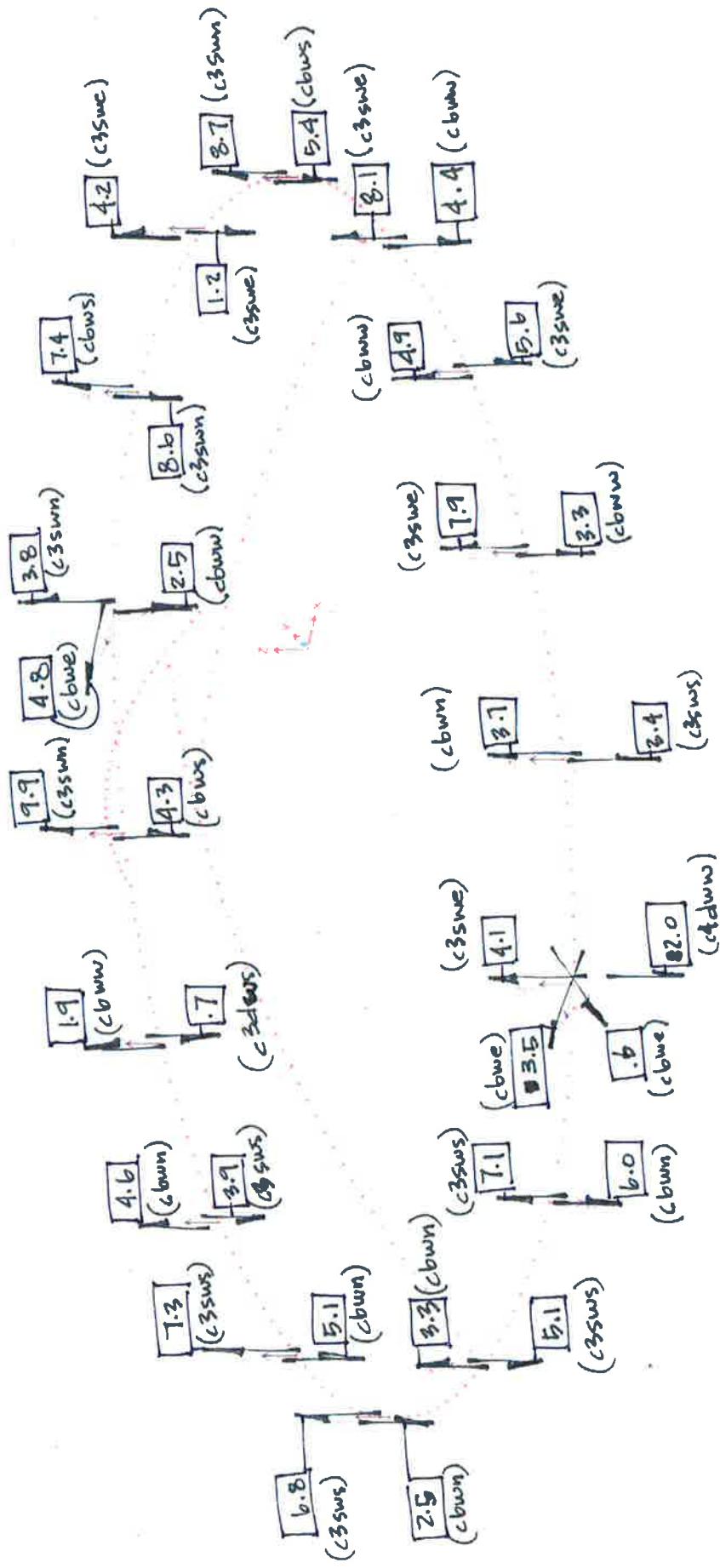
Penetrate  
TS 6x1d x 315"

crosscut  
PBxx  
(medium) (TR)

NAME H

sternal + vertebral system





Steel Frame w/  
controllable reaction  
(+ L.C.E.E.) (H.P.S.)

IMAGE J

• Ultimate loads (in kips)  
• reactions (A unique  
is response to prece  
action & in gravity  
intersection

Parmly ETFE foil skylight system - version D

GRID / akd / ffj

Mar-13

TABLE: Base Reactions

OutputCase	GlobalFX	GlobalFY	GlobalFZ	GlobalMX	GlobalMY	GlobalMZ	XCentroidFZ	YCentroidFZ	ZCentroidFZ
Text	Kip	Kip	Kip	Kip-ft	Kip-ft	Kip-ft	ft	ft	ft
c1	-7.331	-0.548	10.567	-16.466	0.341	-11.323	-0.032	-1.558	0.000
c3swn	-5.246	-0.575	35.093	-51.751	50.066	-9.070	-1.427	-1.475	0.000
c3sws	-5.096	-0.447	37.153	-163.371	80.917	-6.981	-2.178	-4.397	0.000
c3swe	-5.589	-0.500	36.138	-93.785	17.202	-8.452	-0.476	-2.595	0.000
c3sww	-4.793	-0.538	35.865	-120.784	113.579	-7.647	-3.167	-3.368	0.000
c3dwn	-6.131	-0.407	16.654	-73.881	17.503	-10.031	-1.051	-4.436	0.000
c3dws	-5.804	-0.464	17.112	-12.418	14.035	-8.954	-0.820	-0.726	0.000
c3dwe	-5.787	-0.522	14.914	-35.792	36.301	-8.340	-2.434	-2.400	0.000
c3dww	-6.153	-0.355	14.188	-33.952	-6.074	-9.435	0.428	-2.393	0.000
c4swn	-6.293	-0.590	10.133	86.831	-23.514	-11.609	2.321	8.570	0.000
c4sws	-5.992	-0.335	14.253	-136.408	38.188	-7.432	-2.679	-9.571	0.000
c4swe	-6.980	-0.442	12.223	2.763	-89.241	-10.374	7.301	0.226	0.000
c4sww	-5.386	-0.518	11.678	-51.234	103.513	-8.764	-8.864	-4.387	0.000
c4dwn	-6.569	-0.538	4.370	79.915	-33.690	-11.909	7.709	18.286	0.000
c4dws	-6.213	-0.340	7.990	-89.236	17.288	-8.049	-2.164	-11.169	0.000
c4dwe	-7.041	-0.448	5.590	20.886	-83.273	-10.339	14.897	3.736	0.000
c4dww	-5.811	-0.461	4.903	-24.099	66.121	-9.323	-13.485	-4.915	0.000
c6wn	-7.727	-0.652	-1.575	129.572	-55.124	-13.816	-34.990	-82.246	0.000
c6ws	-7.426	-0.396	2.545	-93.667	6.579	-9.639	-2.585	-36.811	0.000
c6we	-8.414	-0.504	0.515	45.504	-120.851	-12.581	234.862	88.433	0.000
c6ww	-6.820	-0.580	-0.030	-8.492	71.903	-10.972	2364.679	279.290	0.000

IMAGE K

## Enclosure Two



grid  
box 30797  
seattle 98113

## TEST REPORT



Alliance Tubular Products LLC  
A PTC Alliance Company  
P.O. Box 2298  
Alliance, OH 44601-0298

BUY  
AMERICAN

S H TEAM TUBE LLC  
I C/O BUCKEYE HONE  
P 1882 BUTLER PIKE  
MERCER, PA 16137

B  
O  
L  
D  
TEAM TUBE LLC  
13821 NE JARRETT ST  
- PORTLAND, OR 97230

The following references are included in this report:

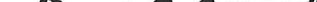
PURCHASER ORDER NUMBER	PTC Order Number	PAGE	FORM
33345	821444	1	48-001

The following tests were successfully performed:  
**NON-DESTRUCTIVE ELECTRICALLY TESTED**

**Killed Steel**

ERW STEEL MECHANICAL TUBES- CD SIZE: 5.500 (139.70) OD x 4.488 (114.00) ID  
SPEC: ASTM A513-12, ST52.3, ERW, TYPE 5, SRA, AW, MECHANICAL TUBING  
GRADE: ST52C / 317MM HT1 STRESS RELIEVE

HEAT NUMBER	PCS.	TOTAL LENGTH SHIPPED	YS- ksi (N/mm <sup>2</sup> )	TS- ksi (N/mm <sup>2</sup> )	E LONG. IN IN <sup>1</sup>	HARDNESS	Y/T
46677D	18	416' 7"	83.5 (576)	102.9 (709)	24%	96 RB	

THIS IS TO CERTIFY THAT THE ABOVE PRODUCTS HAVE BEEN MADE IN THE U.S.A. AND HAVE BEEN INSPECTED AND TESTED IN ACCORDANCE WITH AND HAVE MET ALL REQUIREMENTS OF THE SPECIFICATION.  
EPC ALLIANCE BY 

"THE RECORDING OF FALSE, FICTITIOUS OR FRAUDULENT STATEMENTS OR ENTRIES ON THIS DOCUMENT MAY BE PUNISHED AS A FELONY UNDER FEDERAL STATUTES, INCLUDING FEDERAL LAW, TITLE 18, CHAPTER 47."